

TOWN OF CAMP VERDE

ENGINEERING DESIGN STANDARDS MANUAL



**APPROVED BY TOWN OF CAMP VERDE COUNCIL
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1 ENGINEERING DESIGN STANDARDS

1.1 INTRODUCTION

The Town of Camp Verde has determined that to provide clear concise direction to design professionals, the development community, and contractors, a single point of reference is needed. This manual is intended to apply to new Town infrastructure new subdivisions, and new commercial projects. This manual does not apply to single family homes other than when the home is in a flood hazard area, grading over 100 cubic yards is shown on plans, the local Fire Department has specific requirements, and/or retaining walls are used.

1.1.1 Purpose

The purpose of establishing the EDSM is to provide uniform and functional facilities and infrastructure to ensure the health, safety, and enhance the quality of life for the residents and visitors of Camp Verde.

The EDSM also provides owners, developers, and contractors with directions, construction requirements, and the Town's minimum expectations for the development of infrastructure improvements. The design concepts and specific technical data presented in this Manual are provided to guide and inform the design and development community of the Town's expectations.

This EDSM is not a substitute for sound engineering judgement. The ultimate responsibility for the constructed infrastructure rests with the engineer of record and the general contractor.

1.1.2 Applicability

- A. The EDSM along with the Camp Verde Town Code, the Camp Verde Planning and Zoning Ordinance, Maricopa County Association of Governments (MAG) Standard Details, MAG Standard Specifications, and Town of Camp Verde Standard Details described herein shall govern all infrastructure improvements.
- B. This document is supported by various publications, which comprise the standard references for the types of work encompassed in the Public Works Department. In the event of any conflict or discrepancy between the EDSM and any of the related publications listed below, the EDSM shall take precedence.
- C. The most recent versions of each of the following are considered to be incorporated by reference into this document.
 - 1. Town of Camp Verde General Plan
 - 2. Town of Camp Verde Planning and Zoning Ordinance
 - 3. Camp Verde Municipal Code

4. American Association of State Highway and Transportation Officials (AASHTO)
 - Policy on Geometric Design on Highways
 - Low-Volume Road Design
 - Roadside Design Guide
5. American Society for Testing and Materials (ASTM)
6. American Water Works Association (AWWA)
7. Arizona Revised Statutes (ARS)
8. Arizona Administrative Code (AAC)
9. Institute of Transportation Engineers (ITE)
 - Residential Street Design and Traffic Control
 - Residential Streets
 - Trip Generation: An Informational Report
 - Parking Generation
 - Transportation Research Board (TRB)
 - Highway Capacity Manual
10. Local and Regional Transportation Studies/Plans
13. OSHA Construction Safety Orders

1.1.3 Scope

The Town of Camp Verde EDSM consists of the following Sections below. The EDSM outlines the Town of Camp Verde Design Standards, Construction Specifications, and Standard Details.

1. Introduction
2. Grading
3. Drainage
4. Water
5. Reclaimed Water
6. Wastewater
7. Transportation/Traffic
8. Dry Utilities
9. Survey
10. Alterations & Modifications
11. Record Drawings (As-Builts)
12. Appendix – Town of Camp Verde Standard Details

1.2 REGULATIONS & PLANNING

1.2.1 Town Codes, Ordinances & Regulations

The EDSM clarifies and supplements requirements contained within the Camp Verde Municipal Code, TOCV Planning and Zoning Ordinance, TOCV Subdivision Ordinance, MAG Uniform Standard Specifications and Standard Details for Public Works Construction, Drainage Design Manual for Yavapai County, ADOT, and other local, state, and federal codes and regulations applicable for development within the Town.

The EDSM is not intended to interfere with or annul any other ordinance, rule or regulations, statute, or other provision of law except as provided herein. Where any provision of the EDSM impose restriction different from those imposed by any other provision of law, the provision that is the more restrictive or imposes higher standards upon development and use of land shall govern.

1.2.2 Comprehensive Planning

The Town has adopted a General Plan, as well as several area specific plans as planning documents for future development within the Town. The Town has developed water, sewer, drainage, and transportation master plans to ensure that future infrastructure improvements are achieved through an orderly development of the Town planning areas. Developers and engineers shall review these documents to ensure any proposed new developments or improvements are in concert with the Town's future growth objectives.

1.2.3 Other Regulatory Agencies

Regulatory permits and/or agreements may be required by other State and Federal agencies. This includes but is not limited to Arizona Department of Water Resources (ADWR), Arizona Department of Environmental Quality (ADEQ), Arizona Department of Transportation (ADOT), Yavapai County Environmental Services Department (YCESD), Federal Emergency Management Agency (FEMA), and the United States Army Corps of Engineers (USACE). Approval of any improvement plans of construction activity by the Town does not exempt the project owner from the requirement to comply with the regulations of any of these agencies. Prior to construction of public improvements, the Developer shall provide copies of all such permits to the Community Development Department, or confirmation from the regulatory agency that such permits are not required.

1.3 DEFINITIONS

In the EDSM, the intent and meaning of the terms that are used shall be as defined in Section I of the State Specification except as noted or revised herein. If there is a question on a definition, the Town Engineer should be contacted for the Town's interpretation.

Bedrock: The solid and/or undisturbed in place either at the ground surface or beneath surface deposits of gravel, sand or soil.

Building Official: The Building Official of the Town of Camp Verde Community Development Division.

Clearing & Grubbing: The removal of any plant, bush, tree, cacti, or earth or rock in a defined area.

Contractor: Any person or persons, firm, partnership, corporation or combination thereof, who has/have entered into a contract with any person, persons, corporation, company, special district, or the Town of Camp Verde as party or parties of the second part, or his/her/their legal representatives, for the construction of any public improvement or portion of any public improvement within the Town of Camp Verde or its service area.

Department: The Town of Camp Verde acting by and through its Department of Public Works, including the Director and his/her duly authorized representatives, either employed by or contracting with the department, acting within the scope of the duties delegated to him/her.

Detention: Stormwater Detention refers to the temporary storage of storm runoff and controlling the release so peak flows are mitigated.

Developer: The owner of his/her designated representative, of land where any public improvement is proposed to be constructed.

Director: The Director of the Town of Camp Verde Department of Public Works acting directly or through his/her duly authorized representatives, either employed by or contracting with the Department acting within the scope of the duties delegated to him/her.

Earthwork: The cut, fill, import or disposal of excess earth, sand, gravel, rock or other earthen material.

Engineer: Engineer shall mean Town Engineer of the Department of Public Works acting directly or through his/her duly authorized representatives, either employed by or contracting with the Department and acting within the scope of the particular duties delegated to him/her.

Engineer of Record (EOR): The Project Engineer designated by the Developer as being responsible in charge of civil engineering work as defined in A.R.S. Title 32, Professions and Occupations.

Engineering Design Standards Manual (EDSM): The Design Standards, Construction Specifications, and Standard Details which comprise this volume along with all other standard references incorporated herein.

Excavation: Any act by which earth, sand, gravel, rock, or other earthen material is cut into, dug, uncovered, displaced or relocated, and shall include the conditions resulting there from.

Existing Grade: The grade or elevation of the existing ground surface prior to excavating or filling.

Fill: Deposits of soil, rock, or other materials placed by humans.

Finish Grade: The final grade or elevation of the ground surface after grading is completed.

First Flush: The initial surface runoff of a rainstorm. Stormwater runoff during a First Flush event generally has high concentrations of pollutants.

Grading: Any excavating, filling, moving of natural earthen ground surface or combination thereof, and shall include the conditions resulting from any excavation, fill or movement of the natural earthen ground surface, including the natural drainage pattern.

Laboratory: Any testing agency or testing firm, which has been licensed by the State of Arizona to act in such capacity and meeting the requirements of the Town Engineer.

NPDES/AZPDES: The Federal National Pollution Discharge Elimination System (NPDES) Program administered normally pursuant to 33 U.S.C. Section 1342 and the Arizona Pollution Discharge Elimination System (AZPDES) Program administered in Arizona pursuant to Arizona Revised Statutes, Title 49, Chapter 2, Section 3.1, requiring the control or elimination of erosion and other forms of water pollution from a site.

Public Improvement: Improvements, which will be accepted for operation and maintenance by the Town of Camp Verde, including any subdivision or land use where the improvement is determined to be of sufficient public benefit.

Retaining Wall: A wall, which supports earth higher on one side than the other side (measured from the bottom of the footing to the top of the retaining wall).

Right of Way: The full width of land owned (fee) or controlled (easement) by the Town, upon which the traveled way is constructed, and which usually extends considerably beyond the edge of pavement (or traveled way) to the boundaries of the adjacent private properties.

Right of Way Permit: Authorization by the Department to conduct work, reconstruct or place improvement in established or proposed Town rights of way or easements.

Rough Grade: An approximate elevation of the ground surface conforming to the proposed design.

Service Area: Those parcels located within an area identified by ADWR and ADEQ that receive utility services (i.e. water and/or wastewater) by the Town of Camp Verde.

Site: Any lot or parcel of the land or contiguous combination thereof, under the same ownership, where grading is performed or permitted.

Soil: All earth material of whatever origin that overlies bedrock.

State: When the State Specifications are applicable, the word “State” as used in the State Specifications shall mean the State of Arizona.

State Standard Specification: The latest edition of the ADOT Standard Specifications for Road and Bridge Construction and of the associated Standard Plans (ADOT Roadway Design Construction Standard Drawings).

Stormwater Retention: The capture of stormwater to reduce runoff and/or for the purpose of groundwater recharge.

Town: The Town of Camp Verde (TOCV), Arizona.

Watercourse: a lake, river, creek, stream, wash, arroyo, channel or other topographic feature on or over which waters flow at least periodically. Watercourse includes specifically designated areas in which substantial flood damage may occur (ARS 48-3601).

1.4 AMENDMENTS TO THE DESIGN STANDARDS

Amendments to the Engineering Design Standards may be made by the Town Engineer as needed.

When the Town Engineer determines that an update to the Engineering Design Standards is necessary, approval for the update will depend on the anticipated cost to the developer:

- For updates with an anticipated cost of less than \$500, the Town Engineer may amend the Engineering Design Standards Manual (EDSM) with appropriate director-level approval.
- For updates with an anticipated cost of \$500 or more, the Town Engineer will seek approval from the Town Council before amending the Engineering Design Standards Manual (EDSM).

2 GRADING

2.1 INTRODUCTION

2.1.1 Purpose

- A. The purpose of this Section is to promote health, safety, and public welfare by establishing minimum requirements for regulating grading activities. This Section provides standards, guidelines and design criteria for the design and construction of grading, drainage and site restoration projects within the Town of Camp Verde. Section 2 sets forth rules and regulations to control earthwork construction, including excavation embankments.
- B. This Section is complimentary to the Camp Verde Municipal Code, the Camp Verde Zoning Code, and other Sections of the EDSM.

2.1.2 Applicability

- A. The standards and guidelines of this Section shall apply to any moving, relocating, cutting or filling of earth within the Town of Camp Verde. A grading and drainage plan and permit are required under the following circumstances:
 - 1. For any cut and/or fill of dirt that exceeds one hundred (100) cubic yards (CY) or clearing and grubbing of more than ten thousand (10,000) square feet (SF).
 - 2. A grading permit may also be required for grading which is less than one hundred (100) CY if the work impacts drainage on adjacent upstream and/or downstream properties or structures.
 - 3. No grading shall be performed without first having obtained a permit from the Building Official or Town Engineer. A grading permit does not include the construction of retaining walls or other structures.
- B. The guidelines of this section shall not apply to the following development activities:
 - 1. Selective thinning of vegetation for fire control as approved by the Fire Department.
 - 2. Individual single-family home construction may be exempt, where slopes are less than twenty (20%) percent or where the site will not be subject to mass grading if meeting the requirements above.
 - 3. Grading an isolated, self-contained area, provided that the public is not endangered and that such grading will not adversely affect adjoining properties.
 - 4. Excavation or construction of a structure permitted under this code.
 - 5. Cemetery graves.
 - 6. Refuse disposal sites controlled by other regulations.
 - 7. Excavations for wells, or trenches for utilities.

8. Mining, quarrying, excavating, processing or stock-piling rock, sand, gravel, aggregate or clay controlled by other regulations, provided that such operations do not affect the lateral support of, or significantly increase stresses in, soil on adjoining properties.
9. Exploratory excavations performed under the direction of a registered design professional.

Exemptions from the permit requirements shall not be deemed to grant authorization for any work to be done in any manner in violation of the provisions of this code or any other laws or ordinances of this jurisdiction.

2.1.3 Design Standards & Guidelines

A civil engineer registered in the State of Arizona shall analyze, design, and seal all grading and drainage plans for projects that meet the requirements noted above.

2.1.4 State Regulations

- A. Permits issued under the requirements of this Section shall not relieve the permittee of the responsibility for securing required permits for work to be done that is regulated by any other ordinance, department, or division of the Town of Camp Verde or other governing agency.
- B. All grading work is subject to the requirements of the National Pollution Discharge Elimination System (NPDES) and Arizona Pollution Discharge Elimination System (AZPDES). Before a grading permit is issued on developments greater than 1 acre, the developer or contractor shall submit to the Town Engineer a Notice of Intent (NOI) issued by the Arizona Department of Environmental Quality (ADEQ). The work shall abide by the General Permit and have a Stormwater Pollution Prevention Plan (SWPPP).

2.2 GENERAL INFORMATION

- A. The issuance of a permit or the approval of construction documents and specification shall not be construed to be a permit for, nor an approval of any violation of or deviation from the provisions of this Section or any other ordinance, law, rule or regulation.
- B. No clearing and/or grubbing of a site shall occur without first receiving a grading permit. When clearing and/or grubbing has commenced, the site shall be continuously worked until all infrastructure is completed and all erosion control measures and slope stabilization have been completed.

2.3 GRADING STANDARDS & CRITERIA

2.3.1 Grading Standards

- A. Portions of the site or lot to be disturbed shall be clearly identified on the grading plans as the “disturbed area(s)” or “disturbed area envelope”. Grubbing, grading, and clearing may only occur within the disturbed area(s) identified on the approved grading plan. Portions of the site or lot not identified as disturbed area(s) may not be used for stockpiling materials. All corners of the proposed site shall be clearly identified by a Registered Land Surveyor before any clearing, grubbing or grading begins.
- B. Drainage shall be directed away from all structures per the adopted International Building Code and its applicable adopted appendices.
- C. Residential Development - The maximum height of an exposed vertical cut or fill shall not exceed thirty-five (35') feet unless certification from a Registered Geotechnical Engineer is obtained. The maximum length of an exposed vertical cut or fill shall not exceed one hundred fifty (150') feet from toe to top of slope. See Figure 1 below.
- D. Commercial Development – Commercial development shall not be restricted in terms of height of cut or fill and is subject to the soil stability analysis and scarring mitigation.
- E. All fills shall be compacted to not less than the minimum densities as listed below and as determined by geotechnical engineer

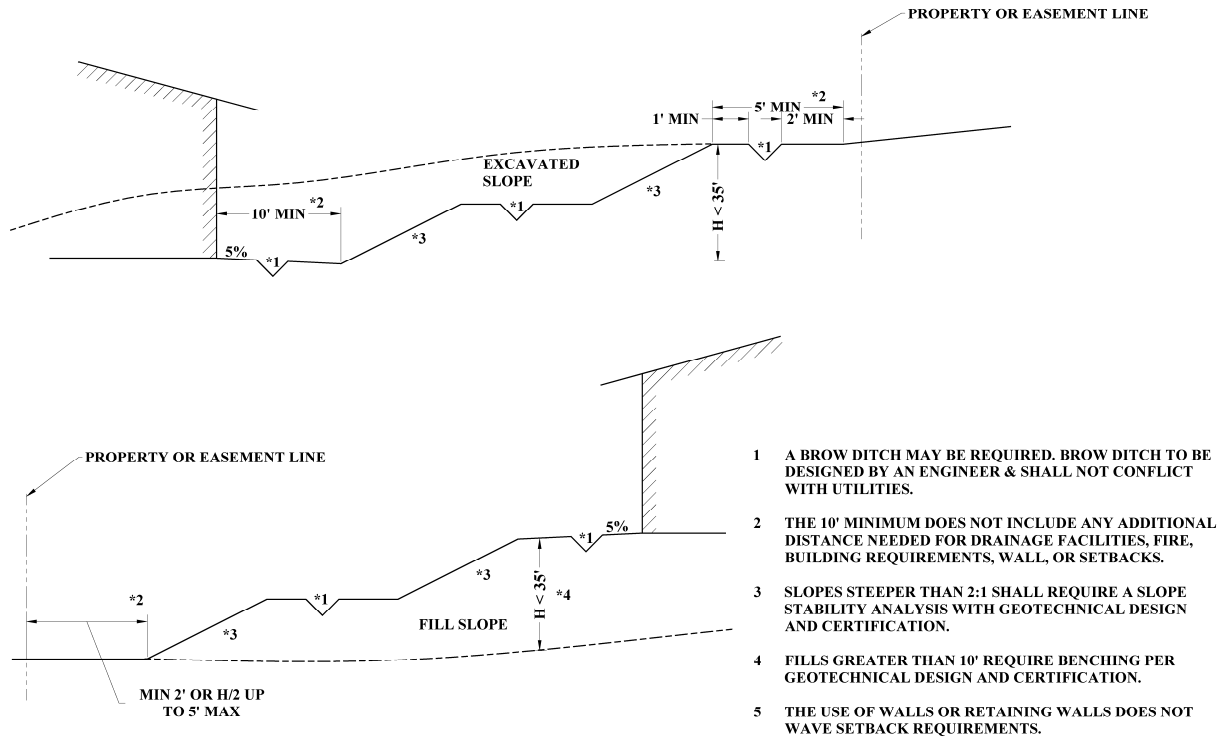
FIGURE 1: FILL COMPACTION

LOCATION	MINIMUM DENSITY
Within the Building Envelope	95%
Parking Lots, Driveways, Roads & Sidewalks	95%
Yards & Open Space	90%

- F. The area over which fills are to be placed shall be cleared of all trash, trees, stumps, timber, and debris of other material not suitable as a foundation for fill. The ground surface shall be prepared to receive fill by removing vegetation, topsoil and other unsuitable materials, and scarifying the ground to provide a bond with the fill material.
- G. Cuts and fills on slopes that encroach into a local drainage tributary (i.e. non-FEMA stream) shall be identified and armored to protect against a 100-year storm event. Analysis shall show conveyance is maintained.
- H. Excess material shall be handled per the Town approved grading plan or removed to a Town approved off-site disposal area.

- I. Where natural areas or preserved vegetation is designated on a site or lot, temporary fencing shall be installed where they abut construction areas to prevent site disturbance of natural areas.
- J. Cut and fill slopes shall be shown on the plans. Slopes steeper than two to one (2:1) (horizontal to vertical) shall require a slope stability analysis prepared by a registered geotechnical engineer. Slope stability analysis for slopes less than two to one (2:1) may be required at the discretion of the Town Engineer.
- K. The top of cut slopes shall be located a minimum of five (5') feet inside the subject property boundary. Buildings located at the bottom of a cut slope shall be located a minimum of five (5') feet away from the hinge point of the cut slope and shall be sloped a minimum of twenty to one (20:1) away from the building.
- L. The toe of fill slopes shall not extend into public ROW or easements. The toe of fill slopes shall be setback inside the subject property line a minimum of two (2') feet or one-half ($\frac{1}{2}$) the fill height up to five (5') feet. Minimum toe setback shall be five (5') feet when height of slope exceeds ten (10') feet.
- M. Buildings located at the top of a fill slope shall be located a minimum of five (5') feet away from the hinge point of the fill slope and shall be sloped a minimum of twenty to one (20:1) away from the building.
- N. Brow ditches shall be provided on the top of slopes to prohibit drainage over and/or down the slopes. The drainage shall be conveyed to a natural or manmade drainage system to minimize erosion.
- O. The natural historical drainage patterns shall be maintained at the property lines.
- P. Slope stabilization is required for all grading work with cut and fill slopes two (2') feet or higher.
- Q. A Landscape Architect or Licensed Landscaper shall prepare stabilization plans for banks greater than eight (8') feet visible from streets and shall submit such plans for Town approval. Where structural designs are required, a registrant shall design and seal the plan.
- R. Retaining walls, visible from streets or nearby neighborhoods, shall be terraced or treated to blend into the natural landscape (for example, tinted concrete, rock facing, slump block, railroad ties, and/or plantings). Unfinished poured concrete and unfinished concrete block walls are not permitted where visible from streets and nearby neighborhoods.
- S. All bedrock cuts higher than eight (8') feet must be included in a geotechnical report, including soil loading calculations, and sealed by a Registered Geotechnical Engineer.
- T. Slopes that are steeper than two to one (2:1) (horizontal to vertical) shall require stabilization to control erosion using approved methods recommended by a soil analysis prepared by a Registered Geotechnical Engineer.

FIGURE 2: RESIDENTIAL GRADING EXHIBIT



2.4 DEVELOPMENT IN WATERCOURSES

2.4.1 Authorization Required for Development (ARS 48-3613)

Except as provided in section 48-3625 and in this section, a person shall not engage in any development which will divert, retard or obstruct the flow of waters in any watercourse without securing written authorization from the board of the district in which the watercourse is located. Where the watercourse is a delineated floodplain, no development shall take place in the floodplain without written authorization from the board of the district in which the floodplain is located.

2.5 PLAN SUBMITTAL REQUIREMENTS

2.5.1 Submittals

The applicant shall complete a grading permit application and provide a Grading and Drainage plan in pdf format to the Camp Verde Community Development Division.

- A. The plans shall include existing contours at a maximum interval of two (2') feet with elevation labels at ten (10') foot intervals, which extend at least one hundred (100') feet beyond the perimeter of the property.
- B. Location of all existing structures, walls (either free standing or retaining), curbs, gutters, sidewalks, streets, all utilities, wells, culverts, channels or other drainage improvements.
- C. Property lines and all existing recorded easements.
- D. Town/County jurisdictional lines.
- E. The 1 percent (1%) annual chance (100-year) FEMA designated Special Flood Hazard Area (SFHA) limits shall be delineated.
- F. Show any hydraulically significant washes on or adjacent to the site.
- G. Proposed culverts.
- H. Location and dimensions of detention basin(s), and drainage structures or improvements.
- I. Finish pad and finish floor elevations, "hinge point" elevations and invert elevations for culverts, channels, and/or drainage swales.
- J. Profile for all culverts, manholes, and inlet structures, showing both existing ground and proposed finished ground. Show inlet and outlet elevations for all culverts, manholes, and inlet structures.
- K. All proposed cut and fill slopes with (horizontal to vertical) slope.
- L. A profile depicting the existing and proposed grading, with all proposed slopes, clearly labeled.
- M. Walls, both free standing and retaining shall have elevations for top of footing and top of wall:

- N. Estimate the cubic yards of material to be moved on the project including both import and export.
- O. Drainage Analysis (Refer to Section 3, Drainage).
- P. A Notice of Intent (NOI) issued by ADEQ.
- Q. A Geotechnical Report prepared by a registered Geotechnical Engineer shall be provided. The report shall identify structural soil properties and recommendations for compaction. The report shall contain soil design criteria for any structures or embankments required to accomplish the proposed grading and where necessary, the report must contain slope stability studies, and recommendations and conclusions regarding site geology.
- R. Projects, which disturb one (1) acre or more or are a part of a larger common plan of development, or subdivision that would result in the accumulated disturbance of one (1) acre or more will require a complete Storm Water Pollution Prevention Plan (SWPPP) binder in conformance with the Arizona Department of Environmental Quality (ADEQ). The SWPPP shall include a Notice of Intent (NOI) from ADEQ and a Sediment and Erosion Control Plan, which includes permanent stabilization and/or landscape plans.
- S. The Engineer of Record (EOR) shall provide the location of any site which provides import to the project or any site to which export is taken. A grading plan or stockpile plan including a SWPPP to control erosion at the off-site location shall be provided for all off-site import/export locations.
- T. Any other items deemed necessary by the Town Engineer.

2.6 GRADING PERMIT

- A. The Grading Permit may be issued when a complete submittal per Sections 2.3 and 2.4 have been submitted, reviewed and approved by the Town Engineer. The issuance of a grading permit shall constitute an authorization to do only that work which is described or illustrated on the permit application, plans and specifications.
- B. Grading Permits are valid for a period of 180 days from the date of issuance. If the work has not commenced within 180 days of the issuance date, or the work has been discontinued for more than 180 from the last “Approved” inspection, the permit will expire.
 - 1. If the permittee is unable to complete the work within the specified time, a request may be made prior to the expiration of the permit in writing, to the Town Engineer for an extension of time. The letter shall set forth the reasons for the requested extension. If, in the opinion of the Town Building Division, such an extension is warranted, the Town Engineer may grant additional time for the completion of the work.
 - 2. If a permit expires, the permit must be re-submitted for review and approval prior to any work commencing. Expired plans and permits shall adhere to the most

currently adopted Camp Verde Municipal Code, EDSM, Camp Verde Planning & Zoning Ordinance, Standard Details, Specifications, and fees.

- C. In granting any permit under this Section, the Town Engineer may attach such conditions as may be reasonably necessary to prevent the creation of a nuisance or hazard to public or private property. Such conditions may include, but shall not be limited to:
1. Requirements for fencing excavations or fills, which would otherwise be hazardous.
 2. Improvement of any existing site condition to bring it up to the standards of this Section.
 3. Temporary fencing of protected hillside and desert preservation areas, prior to commencing grading operations.
- D. Neither the issuance of a permit under the provisions of this Section nor the compliance with the provisions hereof, or with any conditions imposed in the permit issued hereunder, shall relieve any person from responsibility for damage to other persons or property, nor impose any liability upon the Town for damage to other persons or property.

2.7 AT-RISK GRADING

The Town of Camp Verde may consider requests for an At-Risk grading permit on a case-by-case basis and each request will be considered independently on its own merit. Consensus with both the Building Official and the Town Engineer must occur prior to granting an At-Risk grading permit.

2.8 FLOODPLAIN MANAGEMENT

If the site is within a FEMA designated floodplain or floodway, refer to Section 3 - Drainage.

2.9 BLASTING

No blasting shall occur within the Town Limits without a Permit first being issued from the Town Engineer.

2.10 RETAINING WALLS

Retaining walls of any type, exceeding four (4') feet in height, measured from bottom of footing to top of wall, shall require a permit from the Town of Camp Verde Building Division. All retaining walls shall require a design from a Registered Professional Engineer. Retaining walls four (4') feet tall and over are required to have special inspections by a Registered Professional Engineer during the construction process. The special inspection requirements shall apply to work performed under a grading permit.

2.10.1 Permit Documents

- A. A completed grading permit application including wall details. A separate building permit will be required for the construction of any walls.
- B. A site plan showing property lines, existing structures, easements, streets, ROW, required setbacks, and both natural and proposed drainage features.
- C. Retaining Walls:
 - 1. Show location(s), cross section(s), footing, height, finish grade elevations at base and top of wall, backfill and drainage provisions.
 - 2. Structural analysis and design prepared by an Arizona Registered Professional Engineer.
- D. Segmented “dry stacked” Walls:
 - 1. Show location(s), cross section(s), footing, height, finish grade elevations at base and top of wall, backfill and drainage provisions.
 - 2. Include manufacturer’s specifications, design, structural analysis, and construction recommendations.
 - 3. Structural analysis and design prepared by an Arizona Registered Professional Engineer.
- E. Stacked Boulder Wall:
 - 1. Show location(s), cross section(s), footing, height, finish grade elevations at base and top of wall, backfill and drainage provisions.
 - 2. Structural analysis and design prepared by an Arizona Registered Professional Engineer.

2.10.2 Inspections & Testing

- A. The Design Engineer and Geotechnical Engineer shall perform special inspection of the retaining wall construction and compaction testing of the backfill material.
- B. Upon completion of the retaining wall, the Structural Engineer and the Geotechnical Engineer shall submit a stamped sealed certification stating the wall was constructed per the approved civil plans and specifications. The certification shall include the compaction results.

2.10.3 Exceptions

Retaining Walls less than four (4') feet in height that are used for landscaping or decorative purposes with no surcharge shall not require a Structural Analysis or Geotechnical Report.

3 DRAINAGE

3.1 INTRODUCTION

3.1.1 Purpose

The purpose of this Section is to stipulate requirements for the analysis and design of all Stormwater management facilities within the Town of Camp Verde, Arizona. In this context, the term Stormwater includes consideration of all aspects, including capture, conveyance, and quality of such waters.

3.1.2 Applicability

The standards and guidelines of this Section shall apply to the analysis and design of all Stormwater-related facilities for both public improvement and private development and redevelopment projects within the Town of Camp Verde.

3.1.3 Design Standards & Guidelines

Use of this Section does not supersede the need for acquiring various permits or authorizations by the FEMA, EPA, USACE, State of Arizona, Yavapai County, other Town of Camp Verde departments, or any other governing agency having jurisdiction. It is the responsibility of the project owner, project professionals and contractors to comply fully with all applicable regulations.

3.2 DRAINAGE REPORTS & PLANS

3.2.1 Purpose

- A. The purpose of this section is to present criteria for submittal of drainage reports, floodplain studies, grading and drainage plans, and public drainage improvement plans to the Town of Camp Verde for review and approval.
- B. Drainage reports and engineered grading plans are required to:
 - 1. Analyze the impact that the proposed project will have on Stormwater discharges.
 - 2. Provide adequate data to ensure that the project is designed to be protected from flooding and conforms to applicable floodplain and Stormwater management regulations.
 - 3. Provide data for the design of public and private drainage facilities.
 - 4. Show no adverse impact to adjacent and downstream properties.
- C. Drainage reports shall be of sufficient detail to demonstrate that the development or project will not create drainage or flooding problems and that any on-site drainage facilities are properly sized to detain and/or convey the design storm flows. The drainage report must detail that the development of the project will not adversely impact drainage onto neighboring projects or Town infrastructure.

3.2.2 Drainage Report and Plans Required

- A. Drainage reports and Plans shall conform to the requirements, guidelines, and specification of the Drainage Design Manual for Yavapai County

3.2.3 FEMA Designated Flood Hazard Areas

- A. Yavapai County Flood Control District is the Flood Plain Administrator of the Town of Camp Verde. All projects within a FEMA Designated Flood Area will be required to obtain a development permit from Yavapai County Flood Control. Yavapai County Flood Control will determine the Regulatory Flood Elevation for the project. It will be the homeowners/builder's responsibility to have a surveyor establish a benchmark prior to construction to ensure the finish elevation meets the requirements of the development permit.
- B. If the proposed project contains an area that is designated on the currently effective Flood Insurance Rate Map (FIRM) as being in a Special Flood Hazard Area (SFHA), then a detailed floodplain study is required.
- C. The Town of Camp Verde and the Yavapai County Flood Control District will review all floodplain studies for technical compliance and completeness per applicable Town, County, Arizona Department of Water Resources (ADWR) State Standards, and FEMA guidelines. All local and federal review fees associated with map revision requests are the responsibility of the applicant.
- D. Clear Creek - The most recent flood study on Clear Creek was done in 1984. The study was done with minimal information and the Town has found flood levels are higher than shown on FEMA flood maps. The Town recommends a safety factor of 18" added to the Regulatory Flood Elevation assigned by the County Flood Control.

3.3 HYDROLOGY

3.3.1 Purpose

This section provides an overview of hydrologic methods and procedures acceptable for use in the Town of Camp Verde.

3.3.2 General Policies

- A. Pre-development parameters and results shall be consistent with the Town of Camp Verde Area Drainage Master Plan, hydrology results.
- B. Runoff calculations shall be performed for the 2-year, 10-year, 25-year, and the 100-year return periods in conjunction with all drainage reports for private developments and public works projects.
- C. Hydrology for FEMA flood studies in previously unmapped areas shall be in accordance with current FEMA guidelines and State Standards where such differ from Town of Camp Verde development criteria.

3.3.3 Rainfall Data

Typical rainfall values in the Town of Camp Verde are shown in Table 3-1. These values were derived from the rainfall depth-duration frequency statistics for Arizona from NOAA Atlas 14, Precipitation-Frequency Atlas of the United States. When using the NRCS method, the NRCS Type-II 24-hour storm distribution with Antecedent Moisture Condition II is applicable to the Camp Verde area. Other temporal distributions or durations may be accepted.

Table 3-1 is NOAA Atlas 14 Rainfall Data from the specific site of the Camp Verde Town Hall at 473 South Main Street. Rainfall varies within the Town and data is subject to be updated, therefore use site specific data from the NOAA Atlas 14 website for each project.

FIGURE 3: RAINFALL DATA AT CAMP VERDE TOWN HALL

DURATION	STORM FREQUENCY (YEARS)					
	RAINFALL DEPTH (INCHES) / INTENSITY (INCHES/HOUR)					
	2	5	10	25	50	100
5-MIN	0.256	0.349	0.426	0.536	0.628	0.729
	3.07	4.19	5.11	6.43	7.54	8.75
10-MIN	0.391	0.531	0.648	0.816	0.956	1.11
	2.35	3.19	3.89	4.90	5.74	6.66
15-MIN	0.484	0.659	0.803	1.01	1.19	1.38
	1.94	2.64	3.21	4.05	4.74	5.50
30-MIN	0.652	0.887	1.08	1.36	1.60	1.85
	1.30	1.77	2.16	2.72	3.19	3.71
60-MIN	0.807	1.10	1.34	1.69	1.98	2.29
	0.807	1.10	1.34	1.69	1.98	2.29
2-HOUR	0.956	1.27	1.53	1.91	2.23	2.58
	0.478	0.634	0.765	0.957	1.12	1.29
3-HOUR	1.03	1.34	1.60	1.96	2.28	2.62
	0.343	0.445	0.531	0.654	0.758	0.871
6-HOUR	1.25	1.56	1.83	2.23	2.55	2.89
	0.208	0.260	0.305	0.371	0.425	0.483
12-HOUR	1.46	1.79	2.07	2.44	2.74	3.04
	0.121	0.148	0.171	0.203	0.202	0.227
24 HOUR	1.82	2.27	2.64	3.13	3.52	3.92
	0.075	0.094	0.109	0.130	0.146	0.163

3.3.4 Time of Concentration

The Time of Concentration (T_c) may be determined by a wide variety of commonly accepted calculation procedures. Refer to the Drainage Design Manual for Yavapai County (2015).

3.3.5 Detailed Hydrologic Methods

- A. Hydrologic analysis for predevelopment conditions shall be performed using a detailed hydrologic analysis method. Tabular and graphical approximation methods are only acceptable for preliminary work. Other methods may be approved on a case-by-case basis.
 1. Rainfall Losses
 - a. Pre-development condition rainfall losses shall be calculated using a method such as Natural Resource Conservation Service (NRCS), preferred or Green & Ampt. These methods are implemented within commonly

available modeling software such as the U.S. Army Corps of Engineers Hydrologic Modeling System (HEC-HMS) and are compatible with routing methods used for analyzing detention facilities. (Please note that HEC-1 is not an acceptable version).

- b. Input Curve Numbers (CN) for the NRCS method may be determined from soil mapping data available through the NRCS website or based on geotechnical testing of undisturbed native site soils. Further information on selection of CN and implementation of the NRCS method can be found in the NRCS, TR-55 manual.

2. Hydrographs

Synthetic unit hydrograph procedures such as Clark, NRCS, or Kinematic Wave are acceptable.

3.3.6 Rational Method

The Rational Method is acceptable for contributing drainage areas less than one hundred and sixty (160) acres and a T_c less than sixty (60) minutes.

3.4 OPEN CHANNELS

3.4.1 Purpose

- A. This section applies to all private and public open channels. Requirements for roadside ditches are presented in the EDSM, Section 7.
- B. An open channel is defined as a conveyance in which water flows with a free surface. Open channels in the Town of Camp Verde will either be classified as natural or man-made. A channel is considered to be natural if the cross-section geometry, bed slopes, bed and bank materials, vegetative growth, and overbank floodplain areas have not been altered by previous human activity. Any prior or planned alterations to natural channels may trigger any or all of the policies and requirements of this Section.

3.4.2 General Policies for Natural Channels

- A. Developers shall leave natural channels, including the overbank areas within the 1 percent (1%) annual chance (100-year) floodplain, in their natural state.
- B. Natural channels with a 100-year peak discharge greater than fifty (50) cubic feet per second (cfs) shall be analyzed using HEC-RAS to delineate the floodplain boundary.
- C. Drainage easements shall be created for all natural channels and shall include the entire 100-year floodplain.

3.4.3 General Policies for Man-Made Channels

- A. Safety of the public shall be considered in selecting the location and design of man-made channels.
- B. All drainage channels shall include maintenance access and easements to allow periodic maintenance, cleaning and repair.
- C. All channels which will be maintained by the Town of Camp Verde must be within public drainage easements dedicated to, and accepted by, the Town of Camp Verde. The Town does not maintain channels or drainage facilities on private property or within private drainage easements.
- D. Concentrated drainage flows (for all storm frequencies) shall enter and depart from the property to be developed in substantially the same manner and location as under predevelopment conditions, including consideration of depths, widths, and velocities. Substantial deviation from pre-development flow patterns will require written acceptance from affected adjacent property owners and approval by the Town Engineer.
- E. Drainage channels conveying substantial off-site flows may not be constructed within the ROW parallel to public roadways without prior written permission of the Town Engineer. NOTE: This does not apply to roadside ditches intended to intercept local surface runoff from entering the roadway.
- F. No valves, enclosures, transformers, standpipes, poles, mailboxes or other obstructions will be permitted in any open channel with the possible exception of sanitary sewer manholes where allowed by the Town Engineer.
- G. Channel bank soil stabilization and scour protection is required for all channels.
- H. The 100-year peak discharge shall be contained in the defined channel cross section, with freeboard, per this Section.
- I. Minimum finished floor elevations for structures adjacent to channels shall be at least one (1') foot above the 100-year water surface elevation.
- J. Construction activities which will disturb or alter a natural channel may require permits from other government agencies such as the U.S. Army Corps of Engineers or Arizona Department of Environmental Quality to comply with requirements of the Clean Water Act and similar regulatory programs. Approval by the Town of Camp Verde does not supersede or waive compliance with other applicable Federal and State laws.

3.4.4 Channel Design

- A. Established and widely accepted design procedures for opens channels can be found in the current editions of various Federal, State, and County publications. The design engineer shall thoroughly reference and document the procedures and assumptions used and shall adequately demonstrate that the procedure and resultant design is appropriate to the project.

- B. The channel cross section geometry, alignment, and bed slope should result in a stable channel with subcritical flow. Grade control structures in combination with energy dissipation measures may be required to limit slopes and velocities.
- C. Channel side slopes shall be no steeper than three to one (3:1) for vegetated channels and two to one (2:1) for lined channels. Slope stability analysis may be required in some instances.
- D. Channel stabilization measures (including consideration of bank erosion protection, scour protection, and energy dissipation measures) shall be designed for the expected depths, velocities, and shear forces for the 100-year event. The most common channel stabilization methods include turf reinforcement mats, rock riprap, grouted riprap, gabion baskets and mattresses, revetments, concrete, and other biotechnical or synthetic measures. These types of stabilization can be used alone or in combination. Re-vegetation may be acceptable where channel side slopes are three to one (3:1) (horizontal to vertical) or less steep and the underlying soil type is stable for the maximum velocities and forces anticipated.

3.4.5 Freeboard

- A. Freeboard is the minimum additional channel depth required between the calculated 100-year water surface elevation and the top of the lowest channel bank.
- B. Freeboard shall be calculated by a method that includes consideration of depth, velocity and flow regime. The minimum freeboard is one (1') foot.
- C. Additional freeboard may be required at junctions, channel curves, changes in cross section or slope, culvert outlets, where backwater effects may occur, and at locations where supercritical flow or hydraulic jumps occur.
- D. Channel linings of protected open channels shall extend to the elevation necessary to include the freeboard requirement.
- E. Freeboard is not required for minor drainage conveyances such as roadside ditches, yard drainage swales on residential lots, and internal surface drainage for commercial or multi-family developments, where finished floors would not be impacted by overflows.

3.5 CULVERTS & BRIDGES

3.5.1 Purpose

- A. The purpose of this section is to present policies and criteria for the design and construction of roadway culverts and bridges. Culverts consist of a relatively short length of pipe or multiple conduits used to convey stormwater through a roadway embankment and are aligned with open channels. Nationally accepted culvert design procedures are presented in publications such as the (Federal Highway Administration (FHWA) Hydraulic Design Series No. 5 (HDS-5), Hydraulic Design of Highway Culverts NS and HY-8 software.

- B. Bridges are structures that provide a clear span over a watercourse where the full 100-year discharge is conveyed beneath the lowest structural member (low chord) under the deck. Bridges may be required where other Town policies and criteria cannot be met with culverts.

3.5.2 General Policies

- A. All roadway culverts shall be designed to convey at least the 25-year frequency flows without roadway overtopping.
- B. Inlet/Outlet protection shall be provided as necessary.
- C. All culvert designs shall provide a controlled path for overtopping flows. The capacity of that path shall include consideration of clogging factors equal to a fifty (50%) percent conveyance reduction. The overflow path shall be designed to withstand erosion and scour for the calculated overflow depths and velocities.
- D. Culvert pipes shall not be laid on a curved alignment, nor have horizontal or vertical bends except at a manhole or junction box.
- E. The minimum slope for any culvert shall be one half (0.5%) percent but may need to be steeper to prevent siltation for minor storm events such as the 2-year event.
- F. At-grade (ford) crossings for any street classification in new developments are not permitted. They are permissible for private non-commercial driveways.
- G. In new subdivisions where roadside ditches or behind-the-curb drainage is proposed, the design engineer shall determine driveway culvert sizes and shall indicate all such sizes on the subdivision infrastructure plans.
- H. All driveway culverts that are part of an overall residential drainage system shall be designed to convey stormwater runoff for the 25-year event.
- I. The minimum diameter for all roadway crossing culverts shall be eighteen (18") inches. The minimum driveway culvert size is fifteen (15") inches.
- J. Inundation areas on the upstream side of all culvert crossings for the 100-year event shall be contained within drainage easements. Allowable inundation areas and water surface elevations may also be limited by the potential impacts to existing improvements. Ponded water surface elevations shall be one (1') foot below the finished floor of existing structures.
- K. The minimum easement width for culvert pipes forty-eight (48") inches in diameter or less shall be twenty (20') feet. For larger diameter or multiple pipe installations, the easement width shall extend at least eight (8') feet beyond the outside edges of the pipe(s) on each side.
- L. Bridge designs shall be evaluated on a case-by-case basis using nationally accepted design standards consistent with the functional classification of the roadway, and in accordance with Town of Camp Verde EDSM.

3.5.3 Inlet & Outlet Treatment

- A. All culvert pipes shall have an appropriate end treatment.

1. Commercial prefabricated end sections are permitted on culverts thirty-six (36") inches in diameter or less. Riprap protection shall extend around and over the top of the end section a minimum of two (2') feet.
 2. Concrete headwalls are required on all culverts greater than thirty-six (36") inches in diameter within drainage easements and public ROW.
- B. All culvert outlets shall include erosion and scour protection measures that protect the embankment as well as the receiving channel.
1. If the discharge velocity at the end of the pipe is consistent with the normal depth velocity in the receiving channel, then a short riprap apron is adequate to prevent scour at the pipe end that could undermine the headwall or pipe end treatment.
 2. For discharge velocities up to ten (10) feet per second (fps), a rock riprap apron can be used to provide scour protection. The length and width of the apron shall be designed to transition the discharge back to the normal depth and velocity of flow in the channel.
 3. For discharge velocities in excess of ten (10) fps, grouted or wire tied rock in combination with a riprap apron may be necessary to withstand the calculated forces.
 4. For discharge velocities in excess of fifteen (15) fps, energy dissipation structures shall be required in conjunction with additional apron downstream of the energy dissipater to transition the flows back into the receiving channel.
 5. The maximum velocity in any culvert shall be twenty (20) fps.
 6. Roadways designed for overtopping will require additional slope protection on the downstream side of the roadway, since the roadway acts as a spillway.

3.5.4 Culvert Material & Installation

- A. The minimum service life for culverts shall be fifty (50) years for privately maintained facilities and one hundred (100) years for publicly maintained facilities.
- B. The material selection process for culverts shall also include consideration of factors such as hydraulic efficiency, depth of bury, bedding conditions, soil types, sediment loads, abrasion, and corrosion resistance.
- C. Acceptable materials for culverts include:
 1. Reinforced Concrete Pipe (RCP)
 2. Corrugated Metal Pipe (Steel) (CMP)
 3. High Density Polyethylene Pipe (HDPE)
- D. The Town requires reinforced concrete pipe on Town ROW and for all publicly maintained facilities. The Town does not specifically prohibit any particular pipe material for private easements.

- E. Watertight joints are required for all installations where the calculated hydraulic grade line is above the top of pipe at any point along the pipe.

3.6 STREET DRAINAGE

3.6.1 Purpose

When designing streets, the amount of stormwater allowed to flow on the surface of the street is governed by consideration of the need for the road to continue functioning under storm conditions and the safety of all roadway users, including vehicles and pedestrians. When the flow in the street exceeds allowable depth, spread, or velocity criteria, the stormwater shall be intercepted, removed from the roadway into an underground pipe system or roadside ditch, and conveyed to an appropriate, open channel. This section presents the criteria for surface drainage on public and private streets. Publications such as the Federal Highway Administration (FHWA) Urban Drainage Design Manual (HEC-22) present detailed methods for performing calculations related to street drainage.

3.6.2 General Policies

- A. Street drainage and roadways shall be designed to maintain the natural overall drainage patterns existing prior to development, whenever possible. Significant cross-basin diversions that could increase flow downstream to receiving watercourses where capacity and erosion would be adversely impacted are not allowed.
- B. Street drainage design shall consider the 10-year, 25-year, 50-year, and 100-year storms.
- C. When physical conditions do not allow for a sub-surface storm drain system on new curb and gutter street sections, a paralleling surface drainage system may be permitted through the EDSM, Section 10 process.
- D. New inverted crown public streets are prohibited.
- E. Where concentrated flows from adjacent properties enter the road ROW, drainage facilities shall be placed to intercept the runoff at a point outside the street section to prevent those flows from going over sidewalks or curbs, especially in areas where sediment loads are carried.
- F. It is acceptable for adjacent properties to sheet drain onto the streets. However, slopes and grading outside the roadway template shall be designed to intercept and reduce the potential for sediments to be deposited on sidewalks and in the streets.
- G. In all cases, once stormwater runoff enters the street, it shall be confined to the public ROW (on the surface or underground) until it can be discharged in a controlled manner to the appropriate receiving watercourse or other approved conveyance system.
- H. Flow on a local street that has significant depth or velocity approaching an intersection with a collector or arterial shall be captured prior to the intersection. It

is unacceptable for the local street flow to "shoot out" onto the major roadway. Minor and bypass flows that can "turn the corner" may be permitted through the EDSM, Section 10 process.

3.6.3 Street & Gutter

A. Design Frequency & Allowable Spread

1. For Local curbed street sections, runoff from the 10-year design storm must be contained between the curbs of the street. The 100-year flow must be contained within the ROW with a maximum depth at the crown of one (1') foot.
2. For Collector and Arterial curbed street sections, at least one (1) twelve (12') foot travel lane in each direction must remain free from flooding for the 10-year design storm. The 100-year flow must be contained within the ROW, with a maximum depth of eight (8") inches.
3. If either of the above two (2) criteria are exceeded, the excess flow must be captured and removed from the street surface. The subsurface system capacity may need to be increased beyond the 10-year discharge to handle the balance of the 100-year event, which cannot be contained within the ROW, such as in a hillside situation.

B. Gutter Flow

1. The term gutter flow may include the portion of flow that spreads out into the travel lane.
2. Valley gutters perpendicular to the flow of traffic on arterial or collector streets are not permitted. Valley gutters may be used at intersections parallel to the major road. In cases where a valley gutter is not permitted or is inadequate, the water must be removed from the street by storm drains or other approved methods.
3. In locations where superelevated roadway sections are used, gutter flow approaching the transition from the normal crown section shall be reduced to 0.5 cfs to minimize the amount that sheets across the street.

C. Longitudinal & Transverse Slopes

1. The geometric and cross section requirements for streets are specified in the EDSM, Section 7, Transportation – Traffic.
2. Where longitudinal slopes are relatively steep and surface flow velocities are high, allowable gutter capacities shall be reduced to keep depths and velocities at safe levels for pedestrians.

D. Curb & Gutter Terminations

Adequate erosion protection and conveyance measures are required where the curb and gutter section terminates at transitions to uncurbed streets to prevent undermining of the pavement edge and prevent head-cutting of roadway fill slopes.

Such protection will consist of a riprap or concrete spillway that extends to the adjacent roadside ditch or other conveyance.

E. Rural Roads & Roadside Ditches

1. Roadside ditches or channels for rural, uncurbed street sections shall be designed for the 100-year design storm.
2. The underlying soil conditions, flow depths and velocities, and maintenance shall be considered in the roadside channel design. Unlined ditches may be acceptable where sedimentation is expected but are unacceptable where long term erosion is more likely.

3.6.4 Catch Basins

A. Locations

1. Catch basins shall be placed where depth and spread dictate. Bypass flow is permissible on continuous grades.
2. In addition, basins shall also be located in the following locations:
 - a. Where the road sags or superelevates to prevent gutter flow from sheeting across the lanes
 - b. Immediately upgrade of Arterial intersections
 - c. Immediately upgrade of bridges
 - d. Behind curbs, shoulders, or sidewalks to drain low areas to prevent ponding

B. Types

1. Curb opening, grate, and combination catch basin types are all acceptable for use, depending on the location, debris potential, and flow rates.
2. Combination inlets (on-grade) are preferred on public streets due to the reduced clogging tendency when the curb opening is placed upstream of the grate. Calculation of clogging factors for combination inlets can be waived in areas where the likelihood of significant debris is low with approval of the Town Engineer.
3. Curved vane grates are preferred for on-grade locations but are not allowed for sump locations.
4. All grates within the street shall be bicycle friendly.
5. To promote flow capture at catch basins on grade, a two (2") inch gutter depression at the inlet is recommended.
6. Scuppers are acceptable where a receiving ditch, channel, or basin is adjacent and will require a concrete or riprap chute to prevent erosion.

3.7 STORM DRAINS

3.7.1 Purpose

This section presents the criteria for the design of underground storm drain systems that convey street flows once they are intercepted by catch basins. They differ from culverts in that the storm drain system hydraulics are controlled by friction and junction losses whereas culvert hydraulics are controlled predominantly by inlet or outlet conditions.

3.7.2 General Policies

- A. Storm drains shall be designed in conjunction with the street drainage criteria. Accordingly, the minimum design frequency for all storm drain systems shall be the 25-year storm. Storm drain capacity may need to be increased to control the 100-year storm per the street criteria.
- B. Storm drain system outfall points shall be constructed with end treatments, energy dissipation, aprons, scour protection, and other erosion control measures in a manner similar to the requirements for culvert outlets.
- C. If the outlet is perpendicular to the direction of flow in the receiving channel, erosion of the opposite channel bank must be considered, and a channel bank lining of riprap or other appropriate material will be required.
- D. Where storm drain systems are placed within easements, the minimum easement width for pipes forty-eight (48") inches in diameter or less shall be twenty (20') feet. For larger diameter or multiple pipe installations, the easement width shall extend at least eight (8') feet beyond the outside edges of the pipe(s) on each side.
- E. Rubber Gasket Reinforced Concrete Pipe (RGRCP) shall be used for all storm drains within the publicly maintained ROW.

3.7.3 Design Criteria

- A. The minimum allowable slope for any storm drain shall be one half (0.5%) percent.
- B. The minimum velocity for pipes in the system when flowing half full shall be three (3) feet per second (fps).
- C. The maximum velocity in any portion of a storm drain system shall not exceed twenty (20) fps.
- D. In most cases, storm drainpipes shall be on straight runs with uniform slope between manholes or junctions. Horizontally curved storm drains may be permitted through the EDSM, Section 10 process, where it would be beneficial to follow a curved street alignment. The radius of curvature shall not be less than the pipe manufacturer's recommendation. Pulled joints are not an acceptable method.
- E. Minor vertical bends without manholes are discouraged but may be used where necessary to resolve conflicts with other utilities.

- F. Horizontal bends without manholes are acceptable for deflection angles less than five degrees (5°).
- G. The minimum pipe diameter for all laterals shall be eighteen (18") inches, and the minimum diameter for all main lines shall be twenty-four (24") inches.
- H. Storm drainpipe sizes shall increase in the downstream direction.
- I. Watertight joints are required for all installations where the calculated hydraulic grade line is above the top of pipe at any point along the pipe.

3.7.4 Manholes & Junction Structures

- A. The primary function of a storm drain manhole is to provide access to the storm drain system for inspection and maintenance. Junction structures are used where multiple flows converge and are outfitted with a manhole. The terms manhole and junction are used somewhat interchangeably.
- B. At a minimum, manholes are required for the following locations:
 - 1. At junctions where two or more storm drains converge. (Lateral connections from adjacent catch basins do not require a manhole since maintenance access is available through the inlet)
 - 2. At intermediate points along storm drains in accordance with Figure 4
 - 3. At horizontal and vertical deflections greater than five degrees (5°).
 - 4. Changes in pipe size
 - 5. Manholes may also be required by the Town Engineer at other locations to facilitate maintenance

FIGURE 4: STORM SEWER MANHOLE SPACING CRITERIA

PIPE DIAMETER (INCHES)	MAXIMUM DISTANCE (FEET)
24"	300
27" – 36"	400
42" & UP	500

- C. Manholes at vertical deflections shall be at or as close as practical to the point of deflection, with allowance for manufactured bends. If the manhole is not at the point of deflection, it shall be located immediately upstream of the deflection.
- D. Manhole covers should be transversely located within the ROW. Every effort shall be made to avoid locating manhole covers within the vehicle wheel path.
- E. A pressure manhole shaft and pressure frame and cover is required whenever the hydraulic grade line elevation for the design storm at the manhole is less than twelve (12") inches below the manhole cover.
- F. Where the manhole depth is greater than twelve (12') feet the shaft diameter shall be five (5') feet and must be designed to withstand soil pressures. If a manhole will extend below the water table, it must also be designed to withstand hydrostatic pressure and/or seepage.

- G. Manhole shafts shall be five (5') feet in diameter for storm drainpipes thirty-six (36") inches in diameter or greater.
- H. A positive grade to the outflow pipe within the structure is required. A minimum drop of one tenth (0.10') foot is required through all storm drain manholes.

3.7.5 Hydraulics

- A. The Hydraulic Grade Line (HGL) and Energy Grade Line (EGL) shall be calculated for all systems and laterals.
- B. In addition to friction losses, HGL calculations shall include losses at all junctions, manholes, bends, transitions, inlets, and outlets.
- C. Pressure flow is acceptable, but the HGL shall not extend above the ground surface at any point and shall be at least one (1') foot below the opening or grate of a catch basin.
- D. When connecting into an existing storm drain system, the existing system shall be analyzed to determine available capacity. Improvements to the existing system may be required.

3.8 STORMWATER DETENTION

3.8.1 Purpose

This section presents the criteria for the design of stormwater detention facilities in the Town of Camp Verde.

3.8.2 General Policies

- A. Stormwater Detention and Stormwater First-Flush capture is required for all new subdivisions, commercial and industrial developments, re-development of non-conforming sites and other developments that increase impervious area or identifiably impacts adjoining properties.
- B. Stormwater detention within subdivisions shall be vehicular accessible, be on designated tracts, and shall not be situated on private residential property.
- C. Minimum easement width for drainage channels shall be twelve (12') feet.
- D. Detention is not required for a single-family residential structure on a residential lot.
- E. The post-development discharge from a development cannot exceed the total pre-development peak at each discharge point for the 2-year, 10-year, 25-year, 50-year and 100-year storms.
- F. All new developments shall make provisions to retain the First-Flush volume of stormwater equivalent to one-half inch ($\frac{1}{2}$ ") multiplied by the contributing area.
- G. The same hydrologic methodology shall be used for both pre-development and post-development analyses.
- H. Off-site flows shall not be routed through on-site detention.

- I. The use of pavement parking lot storage as the primary detention facility is not permitted unless other sites or detention alternatives are not available. Some inundation of parking lots may be permissible for the major storm event.
- J. Individual on-lot storage systems within single-family residential developments are not permitted for meeting Town of Camp Verde detention requirements.
- K. All detention systems shall include an emergency overflow spillway. The design overflow shall accommodate the post development 100-year peak discharge. The emergency spillway shall not be placed on the impoundment berm.
- L. The minimum finished floor elevation of any structure adjacent to a detention facility shall be a minimum of one (1') foot above the emergency overflow water surface level for the 100-year storm.
- M. Developments which are phased shall prepare a master stormwater detention plan for the entire development. The master stormwater detention plan must either be implemented with the first phase in full, if possible, or detention must stand alone for each phase. Changes to any phase may require an update of the master plan to ensure that all phases remain coordinated.
- N. Discharges from detention facilities shall be designed to enter established downstream drainage systems, (e.g., drainage channels, natural watercourses, public streets, or storm drain systems) whenever possible.
- O. The Town of Camp Verde may accept regional detention basins for operation and maintenance on a case-by-case basis. The Town will not accept local on-site detention basins for operation, maintenance, or liability. No part of a private detention basin shall be constructed in a public ROW or public utility easement.
- P. Maintenance of detention facilities shall be the responsibility of the commercial property owner or the homeowner's association. A maintenance plan shall be prepared in conjunction with the detention basin design that includes both scheduled and unscheduled maintenance activities.

Scheduled maintenance includes such items as mowing, pruning, and trash removal that are performed on a regular basis.

Unscheduled maintenance includes repairs, usually made necessary by storms and floods, which are discovered either during regularly scheduled inspections, or during inspections made after flooding. Unscheduled maintenance shall also include removal of sediment buildup.

Maintenance ramps or other access shall be provided into detention facilities in order to facilitate scheduled and unscheduled maintenance activities. Access easements from public right-of-way shall be provided to all detention facilities. Maintenance of local detention facilities, provided in conjunction with new developments, shall be the responsibility of the commercial property owner or neighborhood association. The Town shall reserve the authority to periodically inspect privately owned detention basins to ensure satisfactory maintenance is being provided.

Final Plats, Development Plans, and CC&R's shall have a note stating that the owner(s) shall be solely responsible for the operation, maintenance, and liability for detention. Failure to maintain detention facilities and drainage channels will result in the Town performing the required maintenance and charging the Commercial Property owner or HOA for work done.

3.8.3 Design Criteria

- A. Hydrologic analysis for detention design shall be performed using a detailed method that is capable of routing a hydrograph based on stage-storage-discharge characteristics and multi-frequency outlet structures. Tabular and graphical approximation methods are only acceptable for preliminary work.
- B. The following minimum slope and depth criteria are required for multi-use basins and basins that have unrestricted access:
 - 1. A maximum of two to one (2:1) (horizontal to vertical) for protected side slopes and three to one (3:1) (horizontal to vertical) for unprotected side slopes where depths of ponding are less than three (3') feet.
 - 2. A maximum of four to one (4:1) (horizontal to vertical) for side slopes where depths of ponding exceed three (3') feet.
- C. Security barriers, with maintenance access gates, are required along the top of all basin side slopes steeper than three to one (3:1) (horizontal to vertical) and where water depths exceed three (3') feet.
- D. A minimum freeboard of one (1') foot above the 100-year high water elevation is required for all detention facilities. Freeboard may include adjacent parking lot areas.
- E. Vegetated embankments shall be less than twenty (20') feet in height and shall have side slopes no steeper than three to one (3:1) (horizontal to vertical).
- F. A geotechnical engineering study and slope stability analysis is required for embankments exceeding ten (10') feet in height or for embankment slopes exceeding those given above.
- G. Top width of the embankment shall be a minimum of one-half ($\frac{1}{2}$) the height of the embankment.
- H. All detention facilities shall incorporate provisions for emergency overflow.
- I. Riprap aprons or other energy dissipating measures should be used at all inflow points (side slope and basin floor) to reduce velocities and encourage sedimentation.
- J. Low flow channels are required on the bottom of basins, which serve as multiuse areas and are recommended on all basins. Low flow channels should be designed with a minimum longitudinal slope of one half (0.5%) percent. Concrete lined low flow channels can be designed with a minimum longitudinal slope of two tenths (0.2%) percent.

- K. Outlet structure designs shall take clogging potential into account, specially designs that utilize orifice plates.
- L. Outlet pipes projecting from a basin side slope or embankment are not permitted. Outlet points shall be constructed with end treatments, energy dissipation, aprons, scour protection, and other erosion control measures in a manner similar to the requirements for culvert outlets.
- M. Where basins outfall near to adjoining private properties, additional measures may be necessary to reduce velocities and allow flows to return to natural conditions, to as great an extent as possible, prior to exiting onto the downstream property.
- N. Drainage crossings or culverts shall be provided whenever runoff entering or exiting a basin crosses pedestrian paths or sidewalks.
- O. The minimum outlet culvert size for detention facilities is twelve (12”) inches. Orifice plates are permitted provided the orifice plate is permanent, tamper-proof, and connects to a twelve (12”) inch minimum diameter outlet pipe. Outlet structures incorporating orifice plates shall include a trash rack to minimize clogging.

3.9 EROSION & SEDIMENT CONTROL

3.9.1 Purpose

- A. The purpose of this section is to control or eliminate soil erosion and sedimentation within the Town of Camp Verde. The Town has established standards and specifications for conservation practices and planning activities which minimize soil erosion and sedimentation in order to accomplish all of the following goals:
 - 1. Protect public health, welfare, and the environment
 - 2. Enable the Town to comply with stormwater pollution prevention provisions of the Arizona Pollutant Discharge Elimination System (AZPDES) Small Municipal Separate Storm Sewer System (MS4) General Permit
 - 3. Properly manage and reduce erosion and the discharge of pollutants from construction sites to the Town’s stormwater system, and in turn, to surface waters and groundwater
- B. This section will present general guidelines and requirements for erosion and sediment control, or Best Management Practices (BMPs) associated with construction activities. These requirements are outlined in the Town of Camp Verde AZPDES General Permit for Stormwater Discharges from Small Municipal Separate Storm Sewer Systems (MS4s) to Waters of the United States.

3.9.2 Applicability

Erosion and Sediment Control Site Plans and/or Stormwater Pollution Prevention Plans (SWPPPs) are required for construction activities that result in the land disturbance of

greater than or equal to one (1) acre, including projects that are less than one (1) acre but are part of a larger common plan or development of sale, that discharge to the MS4.

3.9.3 General Guidelines

- A. If the project disturbs one (1) acre or greater, or is less than one (1) acre but is part of a common plan of development, the owner or operator must apply for coverage under the AZPDES for Stormwater Discharges Associated with Construction Activities. A copy of the Notice of Intent (NOI) to be bound by the State's general construction permit must be filed with the Town during permitting.
- B. The permittee shall implement BMPs as described in the Erosion and Sediment Control (ESC) Plan or SWPPP and shall periodically conduct site inspections to ensure that BMPs are operating correctly and have not been damaged and/or altered.
- C. Sediment control shall be used whenever possible or necessary, to prevent offsite damage or sediment deposition to public streets, ROWs, and stormwater facilities.
- D. All disturbed soils shall be permanently stabilized.
- E. All temporary and permanent erosion and sediment control practices shall be maintained and repaired as needed to ensure continued performance of their intended function.

3.9.4 Design Criteria

SWPPPs, BMPs, Erosion and Sediment Control design methods and facilities shall be in accordance with the Arizona Department of Transportation Erosion and Pollution Control Manual. The SWPPP shall be a separate plan sheet or shall be incorporated into the grading & drainage plans. The SWPPP does not supersede the need for AZPDES or NPDES permits.

3.10 POST CONSTRUCTION STORMWATER TREATMENT

3.10.1 Purpose

This section will present general guidelines and requirements for post construction stormwater management or Best Management Practices (BMPs) for new development and redevelopment projects to prevent or minimize water quality impacts. These requirements are outlined in compliance with the Arizona Pollutant Discharge Elimination System (AZPDES) General Permit for Stormwater Discharges.

3.10.2 Applicability

New development or redevelopment projects that disturb greater than or equal to one (1) acre, including projects that are less than one (1) acre but are part of a larger common plan or development of sale, must capture or treat the "First Flush" as defined in the Yavapai County Drainage Design Manual.

3.10.3 Guidelines

- A. The following categories of structural treatment practices for post-construction stormwater runoff are approved for use within the Town of Camp Verde. In all instances, First Flush Capture/Treatment is required, and Peak Flow discharge shall not increase. Treatment of stormwater runoff must be per specifications of manufacturer and approved by Town Engineer.

1. Inlet Filtration Systems

Inlet filtration systems are accepted for use on project sites with underground storm infrastructure. Inlet filters must be sized and located appropriately to treat the required Water Quality Volume (WQV) for the site. Filter systems shall provide removal of sediments, debris and other nonpoint source pollutants from the site. For sites with a high degree of impervious surface (i.e., parking lots), filters shall provide for the removal of petrochemicals and hydrocarbons.

2. Underground Separators

Underground separators, such as oil-water separators or sand-oil separators, are accepted for use on project sites with underground storm infrastructure. Underground separators may be required for land uses or activities with higher potential pollutant loadings, known as “hotspots”.

3.10.4 Operations & Maintenance

- A. The owner of permanent stormwater management facilities shall be responsible for the proper operation and maintenance of those facilities during and after construction. All permanent on-site BMPs shall be operational prior to the use by any development or phase of development dependent on those BMPs. An Operation and Maintenance Plan shall be prepared for review and approval by the Town Engineer and shall be executed and signed by the Department and the owner.
- B. Development project permits shall be conditioned to require the property owner to execute a Standard Operation and Maintenance Agreement for Post Construction Stormwater Quality Best Management Practices (BMPs) Facilities prior to the issuance of any construction permits. See Standard Operation and Maintenance Agreement below:

STANDARD OPERATION AND MAINTENANCE AGREEMENT

For Post Construction Stormwater Quality Best Management Practices (BMPs) Facilities

Permit Number: _____

Parcel Number(s): _____

Project Name & Address: _____

THIS AGREEMENT, made and entered into this _____ day of _____, 20____, by and between _____, hereinafter called the "OWNER(S)" of the following property and the Town of Camp Verde, Arizona, hereinafter referred to as the "Town";

WHEREAS, the OWNER is proceeding to build on and develop the property; and

WHEREAS, the site plan known as _____ hereafter called the "Plan", which is expressly made a part hereof, as approved or to be approved by the Town, provides for management of stormwater quality within the confines of the property pursuant with the Town of Camp Verde Post Construction Stormwater Runoff Regulation Code and,

WHEREAS, the Town and the Owner, its successors and assigns, agree that the health, safety and welfare of the residents of the Town of Camp Verde, Arizona, require that the on-site stormwater quality BMP facilities be constructed and maintained on the Property; and

WHEREAS, the Town requires that on-site stormwater quality BMP facilities, as shown on the Plan be constructed and adequately maintained by the Owner, its successors and assigns. The Plan is attached hereto and fully incorporated herein as Exhibit A.

NOW, THEREFORE, for consideration of the mutual covenants stated below, the parties agree as follows:

1. The on-site stormwater quality BMP facilities shall be constructed by the Owner, its successors and assigns, in accordance with and specifications identified in the Plan (Exhibit A - attached hereto and fully incorporated herein) and shall, upon construction completion, be certified on the approved As-Built plans by the Engineer of Record.
2. The Owner, its successors and assigns, shall adequately maintain the stormwater quality BMP facilities as outlined in the Plan (Exhibit A) and contained within the Owner's property. Adequate maintenance is herein defined as good working condition, so that these facilities are performing their design functions. Common maintenance shall include the removal of debris (leaves, lawn clippings, twigs, etc.) and trash, checking outlet structures for clogging and cleaning, as necessary, replacing filter media, repairing erosive areas promptly upon observation and removing accumulated sediment.
3. The Owner, its successors and assigns, hereby grant permission to the Town, its authorized agents and employees, to enter the property to inspect the stormwater quality BMP facilities whenever the Town deems necessary. The purpose of the inspection may be to check the facility for proper functioning, to follow-up on reported deficiencies or repairs, to respond to citizen complaints, and/or as deemed necessary

by the Town. If problems are observed, the Town shall provide the Owner, its successors and assigns, copies of the inspection findings and a directive to commence with the repairs within a specified timeline.

4. The Owner agrees to keep written records of inspections and repairs and to provide access to those records to the Town upon request.
5. The terms of this agreement shall be binding upon the heirs, successors and assigns of the Owner and that any subsequent owner of the property shall be responsible for the maintenance of the system.
6. In the event that the Owner, its successors and assigns, fails to maintain the stormwater quality BMP facilities in good working condition acceptable to the Town or make good faith efforts to comply with these requirements, the Owner may be subject to penalties outlined in the Town of Camp Verde Post Construction Stormwater Runoff Regulation Code.
7. The Property Owner(s) agree to defend, indemnify and hold harmless the Town, its' employees, agents, officials, appointees, invitees and volunteers from any claims, demands, causes of action, liability, loss, property damage and/or injury (to property or person, including without limitation wrongful death), whether brought by an individual or other entity, or imposed by a court of law or by administrative action of any federal, state or local government body or agency, in connection with or arising out of the Property Owner's negligence, or intentional acts or failures to act, or contractual breaches arising out of this Standard Operation and Maintenance Agreement.
8. This Agreement shall be recorded by the Town of Camp Verde among the land records of Yavapai County, Arizona with the Yavapai County Recorder's Office and shall constitute a covenant running with the land and shall be binding on the Owner, its administrators, executors, assigns, heirs and any other successors in interests.

WITNESS the following signatures and seals:

Landowner Signature

Print or Type Name

Title

ATTEST:

By: _____

(Type Name)

(Type Title)

State of _____

County of _____

The foregoing Agreement was acknowledged before me this _____ day of _____,
20____,

By _____.

Notary Public

My Commission Expires _____

NOTARY PUBLIC

4 WATER

4.1 INTRODUCTION

4.1.1 Purpose

The goal of the Water System Planning and Design Section is to provide technical management, comprehensive planning, and sound engineering to expand and maintain a reliable and safe water supply, distribution, and storage system. Engineering standards, guidelines, and geometric requirements for the design and construction of public water improvements within the Town of Camp Verde will result in high-quality customer service and fire protection capabilities.

4.1.2 Applicability

The standards and guidelines of this Section shall apply to the construction, modification and extension of all water distribution mains, pump stations and other appurtenances as well as any modification of the public water system to be owned, operated, and maintained by the Town of Camp Verde. Privately funded water mains proposed to be connected to the Town of Camp Verde's system are reviewed for compliance with Town of Camp Verde standards.

Developers are required to install all improvements necessary including off-site improvements to provide water service for domestic and firefighting purposes to their development in accordance with Town Code, this Section, and the most current water study.

4.1.3 Design Standards & Guidelines

New public and private water supply distribution and storage facilities shall be designed in accordance with Section 1.

A civil engineer registered in the State of Arizona shall analyze, design, and seal all proposed water system project plans and specifications that are determined by the Town Engineer to have an impact on the water distribution system. The effects of peak and fire flow demands shall be examined to ensure proper sizing and layout of proposed water system facilities, including impacts to existing offsite facilities where deemed appropriate by the Town Engineer.

4.1.4 State, County, and Town Regulations

- A. ADWR regulations are contained within A.R.S. § 45 and they encompass requirements for water supplies and the management thereof. All new water connections made to the Town's system require confirmation that the supply exists and that there is entitlement to it.

ADEQ's Engineering Manual Bulletin No. 8 "Disinfection of Water Systems" and No. 10, "Guidelines for the Construction of Water Systems" and the Arizona Administrative Code, "Title 18-5-505.B - Approval to Construct", contain specific requirements for submittals, approvals, and notifications when extension of a public water main is proposed. The developer and the Engineer of Record are expected to be aware of and comply with the above-referenced regulations. Additional information can be found on the ADEQ website at www.azdeq.gov/

1. Before civil infrastructure construction permits are issued, the developer shall submit to the Town Engineer an Approval to Construct (ATC) from ADEQ or the local delegated authority.
 2. Prior to the Town Engineer issuing an Approval to Operate (ATO) and release from the Town of Camp Verde, the developer shall deliver to the Town a signed Approval of Construction (AOC) from ADEQ or the local delegated authority
- B. Yavapai County Environmental Services Department (YCESD)
1. Before civil infrastructure construction permits are issued, the developer shall submit to the Town Engineer an Approval to Construct (ATC) from ADEQ through the YCESD delegation.
 2. The developer shall deliver to the Town an Approval of Construction (AOC) from ADEQ through the YCESD delegation prior to opening Town water distribution system to development
- C. Town of Camp Verde
1. Issuance of an Approval to Construct by YCESD or ADEQ does not constitute an approval of plans and reports by the Town of Camp Verde. No construction shall commence prior to the Town of Camp Verde Town Engineer issuing a permit.
 2. An as-built plan submittal is required prior to receiving an "Approval to Operate and Release" from the Town of Camp Verde.

4.2 ADEQUATE WATER SUPPLY DESIGNATION

The Town of Camp Verde holds a Designation of Adequate Water Supply 41-700446.0001 Issued in 2016. The Town is currently modifying the Assured Water Supply & Regional Ground Water Model and is expected to be completed by 2027.

4.3 WATER PLAN APPROVAL

Improvement plans submitted to the Town of Camp Verde for approval shall adhere to professional engineering standards governed by the State of Arizona Board of Technical Registration. Improvement plans that do not meet industry standards or are incomplete shall be returned without review comments.

4.4 WATER SYSTEM REPORTS

All projects shall be required to submit a Water Master Plan Report and/or a Water System Design Report. The purpose of these reports is to provide the Town of Camp Verde with information regarding the potential water demands of the project and verify the capability of the Town of Camp Verde to provide the domestic water use and fire flow demands that will be required. All Water System Reports shall be prepared under the direction of a licensed civil engineer in the State of Arizona. The report shall be sealed and signed by the Engineer of Record.

4.4.1 General Information

- A. Reports shall provide an analysis of the impact the development will have on the Town of Camp Verde's water system. The objectives of the Water Report is to verify the water demand, the system's ability to supply and maintain sufficient fire flows, the available system flows, pressures, and proposed hydraulic regimes. All reports submitted to the Town of Camp Verde for review shall be prepared in accordance with the following guidelines:
 1. Format:
 - a. The report shall be submitted in pdf format
 - b. All reports shall have a table of contents, including page numbers
 - c. A civil engineer licensed to practice in the State of Arizona shall seal each report
 - d. The project name shall be located on the cover
 - e. The name, address, and phone number of the developer/owner and Engineer of Record shall be stated on the cover
 - f. The original submittal date and all subsequent revision dates shall be located on the cover
 - g. Common spreadsheet formats shall be compatible with MS Excel
 2. Existing Conditions:
 - a. Reference any existing master plans, water system model, or design reports applicable to adjacent development
 - b. Include excerpts from existing plans or reports as applicable
 - c. Indicate the dates, times and results of certified flow testing of the existing water system

- d. Include a description based on all parts or reference within a platted development
 - e. Describe the existing topography, and landform features
 - f. Include the location and description of existing utilities in the vicinity
 - g. Describe the existing and proposed site zoning and land uses
 - h. Include reference to elements of the Town of Camp Verde's General Plan and identify any designated character area or studies that will affect the project's design
3. Proposed Conditions:
- a. Summarize the proposed development. Include a site plan that indicates the layout of the proposed development with a survey a minimum of one hundred (100') feet beyond the project limits or as determined by the Town Engineer
 - b. Identify the location and size of all proposed connection(s) to the Town of Camp Verde's water system
 - i. Show all looping and/or extension of water mains into the site
 - ii. Indicate the location of all pressure zone boundary lines
 - c. Provide a looped system for all water supply demands needed to meet the minimum requirements of the Fire Department, maintain water quality standards, and maintain system redundancy for operational flexibility
 - d. Reference which water zone the site is within and address all required fire flows and system pressures
 - e. Address any maintenance responsibilities of the proposed water system
4. Mapping:
- a. All mapping shall conform to Town of Camp Verde CADD standards and the EDSM, Section 9, Survey Controls
 - b. The project name shall be located on each sheet
 - c. The name, address, and phone number of the developer/owner and Engineer of Record shall be stated on each sheet
 - d. The original submittal date and all subsequent revision dates shall be located on the exhibits
 - e. All maps shall be prepared to an appropriate scale that is legible
 - f. All proposed or existing on-site and off-site facilities including, but not limited to, pump stations, transmission and distribution mains, and reservoirs
 - g. All proposed or existing street locations, parcel boundaries, and proposed lots within each parcel
 - h. Label contour lines at two (2') foot intervals

- i. Indicate pressure zone boundaries, pressure-reducing valves (PRV'S), and corresponding zone valves
- j. Include size and pressure settings for all pressure reducing valves (PRV).
- k. Include a vicinity map showing existing and proposed streets to a distance of one mile from the exterior boundaries of the project

4.4.2 Water Master Plan Report Content

- A. When phasing is contemplated or when required by the Town Engineer, a Water Master Plan Report shall be prepared in accordance with the requirements of the EDSM. The report shall also address, but not be limited to the following:
 - 1. The report shall specify the terms and requirements for water service to the development, including any off-site improvements
 - 2. All development project owners shall be responsible for determining their specific water system needs between interim and buildout
 - a. Include the projections for future surrounding developments based on the adopted General Plan Land Use Map to ensure there is no adverse impact on the system
 - 3. A computer water network model, which is compliant with the EDSM, Section 4.4.4 shall be used for the analysis of pressure and flow within the distribution system, verifying that adequate pressures, domestic and fire flows will be available within the proposed development based on projected demands and shall include the following:
 - a. If certified flow tests performed on the system during peak demand to which the project is to be connected do not show that sufficient capacity exists, the computer model will be used to determine the required on-site and off-site facilities such as pump stations, storage tanks, and pipelines necessary to serve the project
 - b. If the proposed development requires a change in zoning that increases density or proposes a water system different from the Town of Camp Verde's Water Master Plan, then additional upstream and downstream, off-site analysis will be required
 - c. Modification of existing or creation of new pressure zones shall require approval of the Town Engineer
 - 4. Master plan mapping shall demonstrate the following:
 - a. Demonstrate compliance with the Town of Camp Verde's Water Model, which encompasses the respective area
 - b. Master planned developments that design a distribution system that will be phased shall provide a synopsis of the phasing with modeling backup to the Public Works Department prior to acceptance of the developments Water Master Plan

- c. Each phase of the master plan must be a standalone document when submitted to the Town of Camp Verde for review of that specific phase

4.4.3 Water Design Report Content

- A. A Water Design Report shall be prepared in accordance with the requirements of the EDSM, Section 4.4.1.
- B. The system design shall comply with the Arizona Department of Environmental Quality (ADEQ), Engineering Manual Bulletin No. 10 and the Arizona Administrative Code R18-5 requirements for system performance and redundancy. The developer shall coordinate water system improvements with the Town of Camp Verde water model and the Capital Improvement Plan.
- C. In addition to the general requirements in the EDSM, Section 4.3.1 and 4.4.1, the Water Design Report shall at a minimum, include the following:
 - 1. Design Documentation
 - a. Note the design compliance with the latest revision of this manual and all other applicable design standards and codes
 - b. Include a narrative of which design procedures, policies and methodologies will be incorporated into the design engineering of the water system
 - c. List the title and version of any software used in the design analysis
 - 2. A summary of the anticipated water demands for the project
 - a. The priority of obtaining water design flows for a non-residential facility is, 1) flow measurements from the actual facility; 2) the use of flows from similar facilities in the immediate area; 3) the use of similar flows from similar facilities in other areas; and 4) flow tables as listed in the EDSM, Section 4, Table 4-1
 - b. Flows may be calculated on a sub-area basis, however, total flows for the entire development shall also be provided
 - c. Flow rates shall include average day, maximum day, peak hour flows and fire flow demands
 - d. Include fire flows and fire sprinkler system demands that will be required for the development in accordance with the Fire Marshal
 - 3. All projects shall be required to provide a fire analysis that shall demonstrate that there are adequate fire flows available from the Town of Camp Verde's existing system to meet the required fire demands of the proposed development, considering the building construction type, height, layout, etc.
 - a. The Engineer of Record shall use a hydraulic model to evaluate the ability of the water distribution system to deliver fire flows to the development
 - b. The developer is responsible for understanding the fire flow requirements of the structures that are to be built and shall ensure

that the water distribution system within the development is capable of delivering the required fire flows

- c. Improvements, including off-site improvements, may be required to meet these criteria
- d. The fire flow demands shall be included in the overall water demand and summarized in the water design report

4. Site Specific Analysis

- a. The fire flow demand shall comply with the International Fire Code (IFC) and the National Fire Protection Association (NFPA), Standard 13, 13R and 13D, Installation of Sprinkler Systems as adopted by the local fire department. The report shall list the applicable codes and standards and the appropriate engineering practices
- b. The “fire flow demand” section shall provide the following information:
 - i. Engineer of Records name, company, address, Town, State, zip code, and phone number
 - ii. Project name, limits, address, general location & proposed uses
 - iii. Detail physical aspects of the proposed on-site structures including:
 - 1. Building height (feet)
 - 2. Number of stories above finished grade
 - 3. International Building Code (IBC) construction type
 - 4. Building area (square feet)
 - iv. The “fire flow” design report shall provide:
 - 1. Revised International Fire Code (IFC) and local Fire Marshal requirements for fire flow
 - 2. Percentage of reduction allowed for the installation of a fire sprinkler system and resultant system demand as approved by the Fire Marshal
 - v. Projects that install fire sprinkler systems shall be in accordance with the Arizona Board of Technical Registration Substantive Policy Statement regarding fire sprinkler systems. The following information shall be included:
 - 1. Range of the fire hazards of the project
 - 2. The hazard classification of the intended occupancy, including any special hazards
 - 3. The appropriate engineering practices
 - 4. The availability and adequacy of the water supply
 - 5. Based on a hazard analysis for the proposed use of each building (including special hazards), the

- appropriate fire sprinkler design density and area of operation shall be provided for each hazard area
 - 6. Anticipated fire sprinkler demand (psi and gpm)
 - 7. Required fire flow and anticipated fire sprinkler demand shall be provided for the project in accordance with the IFC and Fire Marshal
- c. In order to ascertain the availability and adequacy of the water supply for the project, the report shall provide field fire hydrant flow and pressure test results performed in accordance with AWWA M-17 based on a certified test of the existing water system within the previous sixty (60) days. A flow test shall be witnessed by the design engineer and valves operated by Town of Camp Verde Water Division by calling 928.634.0186. The flow test information shall include:
 - i. Test date
 - ii. Test time
 - iii. Test locations
 - iv. Test and residual hydrants
 - v. Orifice size
 - vi. Orifice coefficient
 - vii. Flow test data:
 - 1. Static pressure (psi)
 - 2. Residual pressure (psi)
 - 3. Pitot measurement (psi)
 - 4. Recorded flow rate (gpm)
 - 5. Flow rate (gpm) converted to twenty (20) (psi)
 - viii. Testing technician
 - ix. A copy of the Town of Camp Verde Water Division or Fire Department report showing the flow test was witnessed by Town personnel and/or Fire Marshal.

4.4.4 Water Modeling

A. Water System Model Analysis

1. The Engineer of Record shall provide a model of the planned Water System Master Plan Report or Water System Design Report as delineated in the EDSM, Sections 4.4.1 and 4.4.2. This modeling shall be required where new public water mains are being added, modified or extended in the Town of Camp Verde system. The model shall provide the data necessary to validate that water demands are met to support the project. A water model is also required to demonstrate the required fire flow is adequate in accordance with the adopted Fire Code and Town of Camp Verde Amendments.

2. Infill projects may not require a model. Infill projects include single lot development where fire and domestic flows are taken directly from existing Town of Camp Verde mains and where the zoning is in conformance with the Town of Camp Verde General Plan. If a model is not required, include a narrative of the proposed connections to the Town of Camp Verde.
- B. When water hydraulic modeling information is needed, the following requirements shall be met to deliver model data to the Town of Camp Verde.
 1. Model Software

The Town of Camp Verde currently uses the Bentley WaterCAD software. The modeling software used to evaluate a proposed development does not need to be the same software, but the data shall be provided to the Town of Camp Verde in either WaterCAD or EPA-NET 2.0 format.

2. Model Development

The developer or developer's engineer shall create and utilize a hydraulic model to demonstrate that the proposed infrastructure size, hydraulics, etc. are adequate and satisfy the Town of Camp Verde's requirements for domestic and fire flow demands. The model shall be topographically correct and be based on NAVD 88 Vertical Datum. Representative fire hydrant tests (dual hydrant test with residual and flow pressures witnessed by Fire District Staff) shall be utilized for model calibration. Multiple tests may be required depending on the size and location of development. The Town of Camp Verde may provide specific boundary conditions upon request. The model information shall be delivered to the Town of Camp Verde to be added to the Town of Camp Verde's model, where modeling evaluations may be performed to verify that the proposed infrastructure and demands will not negatively affect the existing TOCV system. Provide calculations documenting full buildout fire flow and maximum day demands and volumes per the Town's standards.

C. Hydraulic Model Data

1. All drawings and model data shall use the Town of Camp Verde coordinate system and datum in the EDSM, Section 9.
2. Model data shall be free of Topology errors in accordance with commonly accepted engineering practices prior to submission to the Town of Camp Verde.
3. Water Distribution System Attributes:
 - a. Mains – Diameter, length, material, location, connectivity with other entities
 - b. Reservoirs – Base elevation, height, volume, location, connectivity with other entities

- c. Pump Stations – Layout, number of pumps, pump curve or design point, elevation, location, connectivity with other entities, pump control scheme
 - d. Nodes (Connection Points) – Location, elevation, demand
 - e. Pressure Reducing Valves and Tank Fill Valves – Location, connectivity with other entities, elevation, number of valves, diameter and valve set points
 - f. Wells – Location, connectivity, design flow, control scheme
 - g. Polygons - Pressure zone boundaries, zoning/density
4. All model data shall include the following:
- a. Demands shall be calculated per EDSM Section 4.4.3.C using average day water demands in gallons per day
 - b. The system shall be capable of providing maximum day demand plus required fire flow
 - c. Verification of the ability of the proposed system to provide adequate flow and pressures under the various demand combinations shall be provided
 - d. Verification that the minimum required pressure throughout the water distribution system is achieved at the highest finished floor elevation or service point in the fire sprinkler line. If the minimum residual pressure under fire flow conditions cannot be achieved, additional private improvements may be required
 - e. Pipes and nodes – Include item identification, demands, elevation, hydraulic grades, length, status, diameter, velocity, head loss/1000 ft
 - f. Reservoirs and pumps – Provide identification, diameter, height, elevation, upstream and downstream hydraulic grade, number of pumps, pump capacity and proposed pump curves
 - g. Pressure Reducing Valves (PRV's) – Provide identification, size, elevation, upstream and downstream hydraulic grade settings
5. Hydraulic Evaluation
- a. New developments that do not constitute infill shall be modeled to determine if the infrastructure is adequate to serve the development and provide the level of service as defined by the Town of Camp Verde's performance criteria.
 - b. The Town will use the Town of Camp Verde's water model to verify the ability of the existing water distribution system to deliver adequate water to the proposed development per the design Engineer's proposal. Supplemental fire hydrant tests shall also be used to make this determination. The developer is responsible to make certain that all fire flow requirements (hydrants and/or sprinklers) within the

development satisfies the Town of Camp Verde EDSM and adopted Fire Code requirements.

- c. The following scenarios shall be modeled:
 - i. Average Day Demands
 - ii. Maximum Day Demands
 - iii. Peak Hour Demands
 - iv. Fire Flow Demands (Which are defined as, the maximum daily demand plus the appropriate fire flow demand)
- d. Model simulations shall be documented in a graphical and tabular format to demonstrate that the water distribution system shall provide the required flow at suitable pressures and water main velocities.
- e. Fire flow modeling results will be documented as follows:
 - i. Junction Report
 - 1. Static Pressures
 - 2. Residual Pressures
 - 3. Demands
 - 4. Calculated Minimum Pressure
 - 5. Elevation
 - 6. Pressure Head
 - 7. Hydraulic grade
 - ii. Pipe Report
 - 1. Length
 - 2. Type
 - 3. Diameter
 - 4. Velocities
 - 5. C-Factor
 - 6. Control Status
 - 7. Discharge
 - 8. Head Loss
 - 9. Head Loss Gradient
- f. Boundary conditions that represent the interface between the development and the rest of the distribution system, shall be explained clearly.

4.5 WATER SYSTEM DEMAND

4.5.1 System Demand

The water system demand describes the standards against which the water infrastructure is measured to determine the criteria of the proposed infrastructure. This water system

demand criteria is based on the Arizona Administrative Code (AAC) and other applicable regulatory standards.

4.5.2 Infrastructure Sizing

Infrastructure shall be sized to supply the maximum daily demand plus fire flow. Fire flows shall be determined by the most currently adopted International Fire Code and identified by the Town Fire Marshal.

4.6 PLANNING

4.6.1 Comprehensive Planning

- A. The Town of Camp Verde has developed a Water System Model to ensure that future improvement requirements are achieved through an orderly development of the Town of Camp Verde planning areas. Any engineer working on the design of public or private water infrastructure shall develop plans consistent with the appropriate sections of the Town of Camp Verde's Model.
- B. Water mains shall be installed along the entire length of the property line frontage of the property being developed. The property line frontage is defined as that portion of a parcel of property that abuts a street, easement, or public rights of way. If a parcel to be developed has more than one (1) frontage, improvements may be required to be installed along all frontages.

4.6.2 Water Facilities

- A. Water facilities (wells, reservoirs, booster pump stations, etc.) are designed and constructed by the Town of Camp Verde through its capital improvement program. Developers needing to construct water facilities shall contact the Public Works Department and request a meeting to coordinate the design of these facilities. The developer shall be prepared to address how the proposed system will conform to the Town of Camp Verde's Water Master Plan. The Town of Camp Verde will address design issues and the review process for these facilities.
- B. Unless otherwise agreed to in writing by the Town Engineer, water facilities shall be located on a tract or lot dedicated to the Town of Camp Verde (conveyed by a general warranty deed) and accompanied by a title policy in favor of the Town of Camp Verde.
- C. Wells
 - 1. ADWR issues the authority to drill groundwater wells both exempt and non-exempt. Before drilling a well, a Notice of Intent to Drill shall be filed with ADWR and a "drill card" must be obtained from ADWR. Non-exempt wells require greater ADWR review and public noticing. Post drilling, and well and pump installation, required well logs and pump completion report must be filed with ADWR.

2. In the event a property has an existing exempt well, and the use continues while a connection is made to the Town's potable water system, a backflow prevention device is required by Camp Verde Town Code, Section 3-6-2 (International Plumbing Code, Amendments).
3. A property owner currently using an exempt well that seeks to discontinue its use for which the Town is able to provide water supplies, then the well owner shall complete ADWR well abandoned requirements. Water service to the property will require a water service agreement to document the volume to be served and conditions of the service. Refer to the adopted Town of Camp Verde Water Management Policy, as extended.
4. Refer to EDSM, Section 4.7.1 for information regarding the drilling of new exempt wells.

D. Reservoirs

Storage facilities shall provide operational storage for peak demands above maximum daily demands, emergency storage for power outages and line breaks, and fire protection storage. Facilities shall be designed to maximize the efficient use of water production wells and pumping facilities. Therefore, storage in each designated service area shall exceed the volume of the sum of 1, 2 and 3 below or one average day demand, whichever is greater.

1. Operational storage equaling twenty (20%) percent of maximum daily demand
2. Fire storage as determined by the fire department ranges from four (4) to ten (10) hours for multiple fire occurrences within a pressure zone. Fire demand rates and storage volumes are determined using criteria established by the American Insurance Association according to the following equation:

$$Q = 1,020\sqrt{P(1 - 0.01\sqrt{P})}$$

P = Population of the Area (in thousands)

Q = Gallons per Minute

3. Emergency storage equaling ten (10%) percent of maximum daily demand
4. The Town Engineer shall determine final sizing and dimensions.

E. Booster Pump Stations

1. Booster pumps shall be designed to maintain adequate pressure for domestic and fire protection water supply. All stations shall provide at a minimum: variable frequency drives, backup power supply, separate electrical room and SCADA compatible with the Town of Camp Verde's current system. Designers shall refer to Engineering Bulletin No. 10, Arizona Department of Environmental Quality, for additional design criteria. The Town

Engineer shall determine final sizing, number of pumps, pump capacity, supervisory control and data acquisition (SCADA) requirements, and building dimensions.

2. A preliminary design report shall be prepared and submitted to the Town Engineer for review by the Utilities Department prior to submittal of final plans for review. This report shall outline the type of equipment and controls proposed, Piping and Instrumentation Diagram (PI&D), and SCADA for the station along with the proposed hydraulics. A final design report prepared by a registered civil engineer licensed in the State of Arizona shall accompany the construction drawings and specifications.

4.7 TRANSMISSION & DISTRIBUTION SYSTEMS

4.7.1 General Information

- A. Effective January 1, 2006 A.R.S 458 45-454 (C) prohibits the drilling of an exempt well (maximum pump capacity of not more than 35 gpm and that is used only for non-irrigation purposes) on land if any part of the land is within one hundred (100') feet of an operating distribution system with a Decision and Order of AWS. For additional detail, see the statute referenced and ADWR Substantive Policy Statement GW43.
- B. The Town of Camp Verde requires water mains to be installed along the entire length of the property line frontage of that property being developed. The property line frontage is defined as, that portion of a parcel of property that abuts a street, easement, or public rights of way. If a parcel to be developed has more than one (1) frontage, improvements may be required to be installed along all frontages if required flow, system redundancy, water quality or paving issues conflict.
- C. Design Policy
 1. The Town of Camp Verde will require the extension of water mains along the frontage, or through a subdivided parcel, to the boundary where future extension of the water main is possible, providing a point of service to adjacent properties, or as determined necessary by the Town Engineer. Reconstruction of residential or commercial structures requires compliance with all current ordinances and design guidelines relating to water main extensions, main sizing, and service lines.
 2. A water system shall be provided as a condition of approval for any subdivision containing lots or parcels less than two (2) acres in area.
 3. In the case of a subdivision with lots equal to or over two (2) acres in area, water supply may be from other than a community system. In this case evidence shall be submitted showing that sufficient potable water is available and can be obtained for all lots in the subdivision.

4. Each lot shall have safe, reliable, and potable water in sufficient volume and pressure for domestic use and fire protection. The Engineer of Record shall verify this by performing a flow and pressure test during peak demand period, of that part of the potable system to be extended or connected onto. The flows and pressure shall meet minimum requirements for domestic and fire flow per applicable regulatory standards. The Engineer of Record shall place a statement verifying this within the design report along with copies of the tests.

D. Design Standards

1. The Engineer of Record shall design proposed water infrastructure in accordance with the design standards listed in the EDSM. These documents contain construction related specifications and details that impact the design of water systems including trenching, bedding, backfill and pavement replacement, etc.
2. Water system expansion and extension requirements to serve both new and existing development are outlined in the Town of Camp Verde Code.

4.7.2 Water Main Pipes & Materials

A. Pipe Sizing

1. The minimum size of a water main for providing fire protection and serving fire hydrants shall be six (6") inches in diameter when service is provided from two (2) directions, or where the maximum length of six (6") inch pipe serving the hydrant from one direction does not exceed fifty (50') feet. A minimum eight (8") inch water main shall be required where service is provided from one direction only. Larger size mains shall be provided as necessary to meet all other design criteria, the development's design report, specific water demands, or the Town of Camp Verde Water System Master Plan.
2. Refer to the adopted Fire Code and Town of Camp Verde Amendments for the maximum length of dead-end fire lines that may be used for fire protection. Fire lines shall be calculated from the street tap to the building for determining sprinkler design.
3. One (1") inch, two (2") inch or three (3") inch water mains are not permitted for new construction as public water mains. Four (4") inch mains may be considered with approval from the Town Engineer.

B. Pipe Materials

1. The use of asbestos cement is prohibited in the Town of Camp Verde Water system.
2. All water mains shall be 350 (CL350) Ductile Iron Pipe (DIP) cement mortar lined and seal coated, or C900 pvc pipe.

3. Service connections for meters shall be one (1") inch minimum, Municipex Water Tubing without splices.
4. Design analysis for wall thickness shall be required in cases where pipelines could be subjected to external loads, which exceed the manufacturer's specifications. These include, but are not limited to, pipelines crossing under storm drain lines greater than thirty-six (36") inches in diameter, pipelines in the roadway alignment that would be exposed to construction vehicle loads prior to paving, and installations exceeding the pipe manufacturer's maximum depth of bury.
5. Polywrap is required to be used on all ductile pipe.

4.7.3 System Layout

- A. To provide appropriate water pressure, water circulation and redundancy, all new water mains shall be designed in a looped configuration where feasible, as determined by the Town Engineer, providing a minimum of two (2) sources that can be isolated by a gate valves, for maximum dead-end line length and size requirements.
- B. Water mains shall not be located within ten (10') feet of a building or retaining wall.
- C. For purposes of horizontal and vertical separation, storm drains and non-potable reclaimed water systems shall be treated as sewer mains. Separation shall be required pursuant to MAG Standard Details 404-1 and 404-2.
- D. Existing water main stubs and service lines adjacent to a proposed development that are not used shall be removed. For mains, this includes removing all valves, capping the tee and removing the pipe at the main. For services, this includes removing the saddle and its appurtenances and installing a full circle stainless steel clamp.
- E. Main Locations:
 1. Public water mains and appurtenances are required to be located within dedicated public ROW or easements
 2. Horizontal Location: All water mains shall be aligned parallel to property lines or street center lines and shall not cross and re-cross the centerline
 - a. Public water mains in commercial, multifamily, and industrial developments shall be located under driveways, or drive aisles and a minimum of ten (10') feet from any structure, including overhangs
 - b. A minimum twenty (20') foot easement shall be provided, and the water main shall be located in the center of the water main easement
 - c. Horizontal alignment changes shall be made with 11 ¼, 22 ½, 45, 90-degree bends. Joint deflection shall not exceed seventy-five (75%) percent of the pipe manufactures specification
 - d. Horizontal and Vertical locations shall be consistent with MAG Standard Detail 404-1

3. Vertical Location:

- a. New water mains, fire lines, and water service lines are not allowed to pass under detention basins
 - i. This does not apply to landscape irrigation lines downstream of proposed backflow prevention devices or buildings
- b. Hydrants, meters, blow-offs and valves shall not be located in washes, detention areas, private driveways, sidewalks, curbs or valley gutters
 - i. Hydrants shall have a maximum bury depth of six (6') feet.

4.7.4 Dead-End Mains

- A. The maximum length for a dead-end water distribution main shall be governed by water quality standards and shall be sufficient to provide minimum pressures and fire flows.
- B. Capped dead-end mains shall be fitted with a flushing device per TOCV Standard Detail 390CV-2. Fire hydrants may be used as a flushing device. Flushing devices shall not be located in washes, detention areas, sidewalks, driveways or paved areas.

4.7.5 Design Flows

- A. The ultimate design flow within the Town of Camp Verde's water transmission and distribution system will be based on the Town of Camp Verde's current water master plan. Water demand for each development will be calculated using the average day demands with appropriate peaking factors applied to ensure that the existing distribution supply is sufficient. Designs will include all necessary improvements, including booster-pumping stations, reservoirs, mains and appurtenances to meet the system's ultimate demand.
 - 1. Hydraulic analysis will demonstrate that the system will provide average day, maximum day demand, and maximum-day demand including fire flow. The average day to maximum day peaking factor is 1.7, and the max day to peak hour factor is 2.0. These factors shall be appropriately increased for restaurants and other high-demand water users.
 - 2. A minimum of twenty (20) psi shall be maintained at the point of maximum fire flow demand during max day plus fire flow conditions. Impacts to surrounding areas shall also be considered.
 - a. Water velocity criteria under maximum day demand conditions are as follows:
 - i. $\text{Velocity} \leq \text{five (5') feet per second (fps) for pipes} < \text{thirty-six (36") inches diameter (Head loss, HL} = < 10 \text{ feet/1,000 feet)}$
 - b. Velocity criteria under maximum day plus fire demand conditions is as follows:

- i. Velocity $\leq$ fifteen (15) fps
- 3. Design flows for all distribution systems will be based upon flow and pressure of the existing system as documented by the Engineer of Record.
- 4. Prior to acceptance by the Town Engineer, all platted subdivisions will conduct an additional flow test at the fire hydrants or flushing devices having the lowest and highest ground elevation within the subdivision.
- 5. Developments that cross pressure zone boundaries shall conduct a flow test within each pressure zone as outlined above. The results of this test, along with a copy of the final plans, shall be submitted to the Town Engineer for review and acceptance.

4.7.6 Fire Hydrant Flow Test Requirements

- A. Pressure and available flow information for existing water mains shall be obtained by having a flow test performed on the system at the developer's expense. Flow tests are required for all commercial projects, multi-family residential projects and public extensions of the Town of Camp Verde's water distribution system. A private fire protection company shall perform the tests in accordance with AWWA manual M-17 and certify the results. The certified flow test results shall be included in all master plans or design reports submitted to the Utilities Department. A copy of the flow test report shall be included when fire sprinkler plans are submitted.
- B. Flow tests may be scheduled through Fire Department and the Town of Camp Verde a minimum of 48 hours prior to the test.

4.7.7 Pressure Requirements for New Development

- A. Pressure extremes in water systems result in the potential for contaminants to enter the network. Low pressures in the water system may allow polluted fluids to be forced into the system, and high pressures may cause ruptures or breaks in the network.
- B. The static pressure in the distribution system shall not exceed one hundred twenty (120) pounds per square inch (psi), and the system shall be designed to maintain a minimum residual pressure of fifty (50) psi at the point of service (box and setter or meter) to be served by system pressure at maximum day demand conditions. The system will be designed to maintain twenty (20) psi minimum pressure under design maximum day plus fire flow requirements. The twenty (20) psi minimum pressure design requirement accounts for the safety factor included in conservative "C" factors to account for aging infrastructure and flexibility in locating pressure zone boundaries.
- C. All distribution water mains, appurtenances and service lines will be designed for a minimum normal internal working pressure of one hundred fifty (150) psi.
- D. Provisions shall be made to protect the system from water hammer pressures. The occurrence and severity of water hammer can be reduced by using pressure-release

valves, surge tanks, variable frequency drives, soft start motor controllers and air chambers.

4.7.8 Pressure Reducing Valves

- A. Approximate pressure zone boundaries and their respective elevations are shown in the Town of Camp Verde's water model and in the 2024 Water System Master Plan. Within the distribution system, or to interconnect to other zones, PRV's shall be required to maintain pressure zones within the distribution system or to interconnect to other zones. Distribution systems will not be designed to operate at pressures in excess of one hundred twenty (120) psi. PRV's shall be built in accordance with TOCV Standard Detail 323CV. A minimum of one (1) PRV in each pressure zone will be designed with a high-pressure relief valve.
- B. Vaults shall be located outside of paved areas adjacent to the back of curb or sidewalk. PRV's shall be located within the rights of way, an easement, or an easement within a private street tract, and shall be provided with unobstructed vehicular access, including parking. Site grading shall route stormwater and discharge water from relief valves away from the vault. Site design shall consider and mitigate any impacts of discharge water on downstream improvements. The location of pressure relief risers shall be shown on the final plans.
- C. The Engineer of Record shall specify in the design report and on final plans, the size of the main and low flow pressure reducing valves, the upstream system pressure, and the design downstream pressure setting.
- D. The Town of Camp Verde requires all metered services to have a pressure-regulating valve installed on the private service line when pressure exceeds 65 psi.
- E. The Town of Camp Verde requires the installation of isolation valves on all PRV stations to facilitate the operation, maintenance, and expansion of the water distribution system.

4.7.9 Valves

- A. The Town of Camp Verde requires the installation of isolation valves to facilitate the operation, maintenance, and expansion of the water distribution system. Water main valves shall meet or exceed the pressure classification of the water main.
- B. Gate valves, required to control the operation of the water system, shall be installed per MAG Standard Detail 301.
- C. All valves shall be resilient seat/wedge gate valves, epoxy-coated inside per the American Water Works Association (AWWA) C-550 per MAG Specification 630.3.

4.7.10 Fittings

- A. Joint deflection shall be discouraged, but where evaluated and allowed for, specific conditions shall not exceed seventy-five (75) percent of the pipe manufactures

specification. Fittings may be required where more than two (2) pipe lengths are deflected.

- B. A minimum distance between fittings shall be greater than twenty-four (24”) inches for constructability. All fittings shall be identified on both the plan and profile view with station and elevations. The Engineer of Record is responsible for verifying the minimum distance necessary for the type and diameter of pipe and related fittings specified for the project.

4.7.11 Valve Locations

- A. Shutoff valves shall be installed on water mains at locations within the distribution system that allow sections of the system to be taken out of service for repairs or maintenance without significantly curtailing service in other areas. Special consideration shall be given to the number of fire hydrants taken out of service. A sufficient number of valves shall be provided on water mains so that inconvenience and sanitary hazards will be minimized during repairs. Valves will be located such that closing no more than four (4) valves can isolate any section of the system.
- B. Valve Location
 - 1. A valve shall be located on each side of a creek, wash, railroad, or highway crossing.
 - 2. Valves shall not be located in curbs, sidewalks, driveways, valley gutters, bike lanes, and vehicle wheel paths.
 - 3. Provide a valve on each fire hydrant lateral and flange the valve to the tee. Provide a valve for all fire protection water supply connections in accordance with adopted Fire Code and Town of Camp Verde TOCV Standard Detail 360CV.
 - 4. The preferred locations of valves shall be set at the intersecting street adjacent to other valves, or at a fire hydrant location.
- C. Maximum spacing of water distribution main valves shall be five hundred (500’) feet.
- D. Install all tees with a valve on the lateral main, so that the lateral can be taken out of service without interrupting the supply to other locations. At intersections of distribution mains valves shall be required on each leg.
- E. All valves require valve boxes installed per TOCV Standard Detail 391CV. All valves shall be installed with a debris cap. The debris cap handle shall be the appropriate color to indicate valve type. Refer to TOCV Standard Detail 391CV.
- F. All valve frame and cover adjustments will be per TOCV Standard Detail 391CV.

4.7.12 Air Release Valves

- A. Air release valves shall be installed at all local high points of water mains four (4”) inches or larger in diameter, as follows:
 - 1. When water main changes from a positive slope to a zero slope or a negative slope

2. Vertical alignment changes that cross under or over another facility, such as other utilities, drainage washes, and the like.
3. All air release valves shall be per TOCV Standard Detail 317CV.

4.7.13 Thrust Restraint

- A. Thrust blocks shall not be allowed for new construction on the Town of Camp Verde's water system unless approved in writing by the Town Engineer. Thrust restraint shall be met by:
 1. Welded joints in steel pipelines
 2. Mechanical joints and/or restraints in pipelines
 3. Locking gasket and ring systems acceptable to the Town Engineer
- B. All valves shall be considered dead ends for thrust restraint.
- C. MAG Standard Details 303-1 and 303-2 include acceptable means of joint restraint for both horizontal and vertical deflections. The Engineer of Record shall verify the water pressures and earth bearing pressures assumed by the details. Mega-Lug Mechanical Joint Restraints shall be required per MAG 303CV-2 on all horizontal and vertical bends.
- D. Couplings, Joints, Gaskets, and Flanges
 1. Couplings, joints, gaskets, and flanges shall conform to Section 610.10 of the MAG Uniform Standard Specifications or equal, unless otherwise approved.
 - a. Thrust Restraint: Joint restraint shall be used at all bends, elbows, tees, crosses, dead ends, stubs, curb stops, fire hydrants, taps, and valve locations on water mains where water flow changes direction or is stopped. The joint restraint limits shall be shown on the plan and profile view. Restrained joint analysis shall be prepared and submitted when necessary, keeping in mind that concrete thrust blocks are not to be considered in the analysis.
 - b. Joint restraint shall comply with MAG Standard Detail 303-1 and 303-2.

4.7.14 Electronic Markers

- A. Tracer wire shall be taped on top of all public water mains, service connections and fire lines. All splices of trace wire shall be Copperhead Snakebite or 3M DBR moisture displacement connectors.
- B. A continuity test will be performed at the completion of installation. Passing test results shall be provided for all pipe segments within the Engineer of Record's as-built data and plan set.
- C. Trace wire stations shall be required on all mains without sufficient valves and other points necessary to access the wire to maintain accurate signal strength.
- D. Maximum distance between test stations shall not exceed five hundred (500') feet.

4.7.15 Pipe Cover

- A. Water mains shall be installed to minimum and maximum depths measured from the proposed finished grade to top of pipe as follows:
 - 1. Water main ten (10") inches and under shall have a minimum cover of three (3') feet. Water Mains greater than ten (10") inches shall have a minimum cover of four (4') feet.
 - 2. Public water mains that are installed through undeveloped property (i.e., locations where the final finished grade elevation is not known, particularly along future street alignments), shall have a minimum cover of five (5') feet from the existing grades.
 - 3. Subsequent fills that produce a cover depth in excess of six (6') feet shall require the main to be vertically realigned within the four to six (4'-6') foot envelope
 - 4. Maximum cover over water mains shall be six (6') feet, except at vertical re-alignments necessary to avoid conflicts with other utilities and storm drains
- B. If a water main is installed within an area to be filled at a later time, adequate pipe protection shall be provided. This may include a temporary berm or constructing the water main to a minimum cover below existing grade. The Engineer of Record shall notify the Public Works Department of such occurrences and address them in the design report or master plan and civil plans.

4.7.16 Wash Crossings

- A. All wash crossings will be constructed using restrained joint Class 350 (CL350) Ductile Iron Pipe (DIP). Bury requirements to place water mains under washes or channels shall be based upon the scour depth of the 100-year peak design discharge (Q100) in the channel or wash. The minimum depth of bury below the design flow line of the channel or wash shall be per EDSM, Table 6-7, unless site specific geotechnical and hydraulic data is provided.
- B. Wash crossings with a one hundred (100) year flow above five hundred (500) cubic feet per second (cfs), shall have the scour depth estimated using Arizona State Standard Attachment SSA 5-96, Guideline 2, Level I, as published by the Arizona Department of Water Resources. The Engineer shall estimate the depth of scour and design the top of pipe to be three (3') feet below the estimated scour depth. The design engineer shall provide a detailed analysis of the scour depth with final plans for review and approval.
- C. All pipelines that must be located within the scour zone or do not meet the minimum required depth of bury, as indicated above, shall be protected by installing a cut-off wall, and energy dissipation downstream of the pipeline to stabilize the scour depth to a minimum of three (3') feet above the pipeline. The design engineer shall design the cut-off wall energy dissipation and include details on the

improvement plans. The Town Engineer shall review pipe protection and scour stabilization requirements on a case-by-case basis.

4.7.17 Separations & Vertical Realignment

- A. For the protection of the public water supply from contamination the Engineer of Record shall maintain separation distances in accordance with the Arizona Department of Environmental Quality Engineering Bulletin 10, MAG Specification Section 610.5 and MAG Standard Details 404-1 and 404-2 Water and Sanitary Sewer Separation/Protection.
- B. Vertical separation of water, reclaimed water, and sanitary sewer mains shall comply with MAG Standard Detail 404-1.
- C. Water mains crossing over culverts and storm drains shall maintain both a minimum of twenty-four (24") inches vertical separation and the minimum depth of bury. If the design cannot provide these clearances, a vertical realignment may be required.
- D. Separation from Storm Drains and Culverts: Water mains shall maintain four (4') feet horizontal and two (2') feet vertical separations from storm drains and culverts. Water mains crossing less than two (2') feet below a storm drain or culvert shall require additional protection such as the use of restrained joint DIP or placing the water main in a casing pipe.

4.7.18 Service Lines & Meters

- A. The water service line and meter shall be sized based upon the total daily demands for the development and the recommended maximum capacity. The minimum size service tap, saddle, line and meter set shall be one (1") inch.
- B. That portion of the water service from the water main up to and including the brass nipple downstream of the meter is public and shall be maintained by the Town of Camp Verde as shown in TOCV Standard Detail 316CV. That portion of the water service beginning at the curb stop after the meter is private and shall be maintained by the property owner. Design of the private on-site portion of the water service shall comply with the current plumbing code and shall include a customer shutoff and pressure-regulating valve. Refer to TOCV Standard Detail 316CV.
- C. Due to the Town of Camp Verde's water billing rate structure, meter sizes shall not exceed the size of the service. (e.g. a one and one half (1 ½") inch meter shall not be allowed on a one (1") inch service). Extra attention is required when sizing services for custom home lots where demands occasionally necessitate meter sizes exceeding one (1") inch.
- D. Service lines are necessary to meet domestic, fire and irrigation demands. Residential fire sprinkler and irrigation demand is normally supplied through the domestic service line and meter. Commercial developments may use separate meters for building and landscape service but will provide separate lines for fire protection.

- E. Installation of metered one (1") inch to two (2") inch water services shall be in accordance with TOCV Standard Detail 316CV. Installation of three (3") inch to six (6") inch metered services shall be in accordance with TOCV Standard Detail 321CV.
- F. Final plans shall show locations of service lines and meters to each unit referenced with stations and dimensions, or offsets, from the street centerline or monument line. Service lines and meter boxes shall be located within a public ROW or a utility easement per TOCV Standard Detail 316CV.
- G. Service taps are not permitted on a fire line, which is designed to service private fire sprinkler systems and/or a fire hydrant.
- H. Water services connections shall be designed perpendicular to the main. Lines shall be continuous from the main to the meter with no splices. Water service lines shall have six (6') feet minimum horizontal separation from sanitary sewer service lines.
- I. No service connections or fire protection systems shall be made directly to water transmission mains.
- J. All non-Municipex service lines which are exposed during construction shall be replaced in their entirety with one (1") inch minimum Municipex Water Tubing. This includes the replacement of service saddles, box and setter and corporation stop in all cases. Refer to TOCV Standard Detail 316CV.
- K. The Developer shall install all water services, meter setter and meter boxes in new development projects.
 - 1. Water meter boxes shall be installed out of traffic areas and behind back of curb and sidewalk. Refer to TOCV Standard Detail 316CV.
 - 2. Metered Services:
 - a. Subdivision plans shall provide a typical water service, location detail
 - b. Where water services are being designed for isolated locations, provide stationing and offset to property line
 - c. Minimum water service size is one (1") inch with a five-eighths ($\frac{5}{8}$ ") by three-quarter ($\frac{3}{4}$ ") inch meter
- L. Existing water services not used will be noted on the approved civil plans and the as-built plan as abandoned at the main with full circle stainless steel repair clamp.

4.7.19 Backflow Prevention & Cross Connection Control

- A. The installation of Backflow Prevention Assemblies is required for all developments with the exception of single family residential unless the property is served by an alternate source of water. The following types of service connections shall require backflow prevention to be installed: commercial meters, master metered systems, irrigation meters, construction meters, multifamily residential meters, and fire protection connections.

- B. Residential uses are required to have a Backflow Prevention Assembly when a well, pond, submerged make up line, automatic irrigation system or fire sprinkler is present, which may raise the degree of hazard for the site.
1. The type of backflow assembly required is determined by the degree of hazard. Reduced Pressure Zone Assemblies (RPZA) shall be required for all commercial, multifamily residential, master metered systems, private potable water systems and irrigation meters. A testable Double Check Valve Assembly (DCVA) shall be required for all fire protection connections.
 2. The Town of Camp Verde Backflow Prevention Program, Camp Verde Municipal Code, Title 14, requires that Backflow Prevention Assemblies be installed according to TOCV Standard Details 316CV or 324CV-1 as applicable for size and in accordance with the adopted plumbing code.
 3. All backflow prevention assemblies shall be approved by the “Foundation for Cross Connection Control and Hydraulic Research of the University of Southern California”, and the Town of Camp Verde Public Works Department.
 4. Initial certification testing is required upon establishment or restoration of water service to the meter. A certified backflow prevention device tester, approved by the Public Works Department, shall make all testing, maintenance and repairs to the backflow prevention assembly at the water service customer’s expense. The initial testing and the required annual testing are the responsibility of the water service customer.
 5. Fire protection systems require the installation of a Town of Camp Verde approved testable double check valve backflow prevention assembly (DCDA) installed at the property line in an insulated enclosure. Refer to TOCV Standard Details 324CV-2, or 324CV-3.
- C. Every effort shall be made to locate the water meter and vault in an area that can accommodate a properly installed backflow assembly. When the location of a backflow preventer cannot be accommodated adjacent to the meter, the designer shall request in writing to separate the backflow assembly from the meter with a detailed explanation why the applicable standard cannot be met as well as the proposed alternative location and measures that will be taken to ensure no taps can be made between the meter and the device

4.7.20 Tract & Easement Requirements

- A. All public water mains and appurtenances located outside of a public ROW or a private street tract with easement overlay shall be placed in the center of a minimum twenty (20’) foot wide easement. The water main shall be located a minimum of ten (10’) feet from the edge of easement. The easement shall have legal access from a public ROW, shall be free of obstructions, shall not be located in a fenced area, and will be accessible at all times to Town of Camp Verde service equipment such as trucks, backhoes, and the like.

- B. Any re-vegetation within the easement shall consist of low-growing shrubs or plant material acceptable to the Town Engineer. Trees may be located along the edge of the easement but not within ten (10') feet of the water main as measured to the trunk of the tree.

4.8 FIRE PROTECTION

4.8.1 General Information

- A. The acting Fire Marshal is the governing authority for the Town of Camp Verde Fire Requirements.
- B. For information related to the most currently adopted plumbing and fire codes, see Fire Protection Standards website ccfmd.az.gov/crr.
- C. See Fire-Flow Requirements for Buildings – Appendix B from the 2018 International Fire Code below for fire-flow requirements.

4.8.2 Fire Flow Requirements

APPENDIX B (from the 2018 International Fire Code)

FIRE-FLOW REQUIREMENTS FOR BUILDINGS

SECTION B101 GENERAL

B101.1 Scope. The procedure for determining fire-flow requirements for buildings or portions of buildings hereafter constructed shall be in accordance with this appendix. This appendix does not apply to structures other than buildings.

SECTION B102 DEFINITIONS

B102.1 Definitions. For the purpose of this appendix, certain terms are defined as follows:

FIRE FLOW. The flow rate of a water supply, measured at 20 pounds per square inch (psi) (138 kPa) residual pressure, that is available for fire fighting.

FIRE-FLOW CALCULATION AREA. The floor area, in square feet (m²), used to determine the required fire flow.

SECTION B103 MODIFICATIONS

B103.1 Decreases. The *fire code official* is authorized to reduce the *fire-flow* requirements for isolated buildings or a group of buildings in rural areas or small communities where the development of full *fire-flow* requirements is impractical.

B103.2 Increases. The fire code official is authorized to increase the fire-flow requirements where conditions indicate an unusual susceptibility to group fires or conflagrations. An increase shall be not more than twice that required for the building under consideration.

B103.3 Areas without water supply systems. For information regarding water supplies for fire-fighting purposes in rural and suburban areas in which adequate and reliable water supply systems do not exist, the *fire code official* is authorized to utilize NFPA 1142 or the *International Wildland- Urban Interface Code*.

SECTION B104 FIRE-FLOW CALCULATION AREA

B104.1 General. The *fire-flow calculation area* shall be the total floor area of all floor levels within the *exterior walls*, and under the horizontal projections of the roof of a building, except as modified in Section B104.3.

B104.2 Area separation. Portions of buildings that are separated by *fire walls* without openings, constructed in accordance with the *International Building Code*, are allowed to be considered as separate *fire-flow calculation areas*.

B104.3 Type IA and Type IB construction. The *fire-flow calculation area* of buildings constructed of Type IA and Type IB construction shall be the area of the three largest successive floors.

Exception: *Fire-flow calculation area* for open parking garages shall be determined by the area of the largest floor.

SECTION B105 FIRE-FLOW REQUIREMENTS FOR BUILDINGS

B105.1 One- and two-family dwellings, Group R-3 and R-4 buildings and townhouses. The minimum *fire-flow* and flow duration requirements for one- and two-family *dwellings*, Group R-3 and R-4 buildings and *townhouses* shall be as specified in Tables B105.1(1) and B105.1(2).

B105.2 Buildings other than one- and two-family dwellings, Group R-3 and R-4 buildings and townhouses. The minimum *fire-flow* and flow duration for buildings other than one- and two-family *dwellings*, Group R-3 and R-4 buildings and *townhouses* shall be as specified in Tables B105.2 and B105.1(2).

B105.3 Water supply for buildings equipped with an automatic sprinkler system. For buildings equipped with an *approved automatic sprinkler system*, the water supply shall be capable of providing the greater of:

1. The *automatic sprinkler system* demand, including hose stream allowance.
2. The required *fire flow*.

TABLE B105.1(1)
REQUIRED FIRE FLOW FOR ONE- AND TWO-FAMILY DWELLINGS, GROUP R-3 AND R-4 BUILDINGS AND TOWNHOUSES

FIRE-FLOW CALCULATION AREA (square feet)	AUTOMATIC SPRINKLER SYSTEM (Design Standard)	MINIMUM FIRE FLOW (gallons per minute)	FLOW DURATION (hours)
0–3,600	No automatic sprinkler system	1,000	1
3,601 and greater	No automatic sprinkler system	Value in Table B105.1(2)	Duration in Table B105.1(2) at the required fire-flow rate
0–3,600	Section 903.3.1.3 of the <i>International Fire Code</i> or Section P2904 of the <i>International Residential Code</i>	500	$\frac{1}{2}$
3,601 and greater	Section 903.3.1.3 of the <i>International Fire Code</i> or Section P2904 of the <i>International Residential Code</i>	$\frac{1}{2}$ value in Table B105.1(2)	1

For SI: 1 square foot = 0.0929 m², 1 gallon per minute = 3.785 L/m.

TABLE B105.1(2)
REFERENCE TABLE FOR TABLES B105.1(1) AND B105.2

FIRE-FLOW CALCULATION AREA (square feet)					FIRE FLOW (gallons per minute) ^b	FLOW DURATION (hours)
Type IA and IB ^a	Type IIA and IIIA ^a	Type IV and V-A ^a	Type IIB and IIIB ^a	Type V-B ^a		
0-22,700	0-12,700	0-8,200	0-5,900	0-3,600	1,500	2
22,701-30,200	12,701-17,000	8,201-10,900	5,901-7,900	3,601-4,800	1,750	
30,201-38,700	17,001-21,800	10,901-12,900	7,901-9,800	4,801-6,200	2,000	
38,701-48,300	21,801-24,200	12,901-17,400	9,801-12,600	6,201-7,700	2,250	
48,301-59,000	24,201-33,200	17,401-21,300	12,601-15,400	7,701-9,400	2,500	
59,001-70,900	33,201-39,700	21,301-25,500	15,401-18,400	9,401-11,300	2,750	
70,901-83,700	39,701-47,100	25,501-30,100	18,401-21,800	11,301-13,400	3,000	3
83,701-97,700	47,101-54,900	30,101-35,200	21,801-25,900	13,401-15,600	3,250	
97,701-112,700	54,901-63,400	35,201-40,600	25,901-29,300	15,601-18,000	3,500	
112,701-128,700	63,401-72,400	40,601-46,400	29,301-33,500	18,001-20,600	3,750	
128,701-145,900	72,401-82,100	46,401-52,500	33,501-37,900	20,601-23,300	4,000	4
145,901-164,200	82,101-92,400	52,501-59,100	37,901-42,700	23,301-26,300	4,250	
164,201-183,400	92,401-103,100	59,101-66,000	42,701-47,700	26,301-29,300	4,500	
183,401-203,700	103,101-114,600	66,001-73,300	47,701-53,000	29,301-32,600	4,750	
203,701-225,200	114,601-126,700	73,301-81,100	53,001-58,600	32,601-36,000	5,000	
225,201-247,700	126,701-139,400	81,101-89,200	58,601-65,400	36,001-39,600	5,250	
247,701-271,200	139,401-152,600	89,201-97,700	65,401-70,600	39,601-43,400	5,500	
271,201-295,900	152,601-166,500	97,701-106,500	70,601-77,000	43,401-47,400	5,750	
295,901-Greater	166,501-Greater	106,501-115,800	77,001-83,700	47,401-51,500	6,000	
—	—	115,801-125,500	83,701-90,600	51,501-55,700	6,250	
—	—	125,501-135,500	90,601-97,900	55,701-60,200	6,500	
—	—	135,501-145,800	97,901-106,800	60,201-64,800	6,750	
—	—	145,801-156,700	106,801-113,200	64,801-69,600	7,000	
—	—	156,701-167,900	113,201-121,300	69,601-74,600	7,250	
—	—	167,901-179,400	121,301-129,600	74,601-79,800	7,500	
—	—	179,401-191,400	129,601-138,300	79,801-85,100	7,750	
—	—	191,401-Greater	138,301-Greater	85,101-Greater	8,000	

For SI: 1 square foot = 0.0929 m², 1 gallon per minute = 3.785 L/m, 1 pound per square inch = 6.895 kPa.

a. Types of construction are based on the *International Building Code*.

b. Measured at 20 psi residual pressure.

TABLE B105.2
REQUIRED FIRE FLOW FOR BUILDINGS OTHER THAN ONE- AND
TWO-FAMILY DWELLINGS, GROUP R-3 AND R-4 BUILDINGS AND TOWNHOUSES

AUTOMATIC SPRINKLER SYSTEM (Design Standard)	MINIMUM FIRE FLOW (gallons per minute)	FLOW DURATION (hours)
No automatic sprinkler system	Value in Table B105.1(2)	Duration in Table B105.1(2)
Section 903.3.1.1 of the <i>International Fire Code</i>	25% of the value in Table B105.1(2) ^a	Duration in Table B105.1(2) at the reduced flow rate
Section 903.3.1.2 of the <i>International Fire Code</i>	25% of the value in Table B105.1(2) ^b	Duration in Table B105.1(2) at the reduced flow rate

Section B106
Referenced Standards

ICC	IBC-18	International Building Code	B104.2
ICC	IWUIC-18	International Wildland-Urban Interface Code	B103.3
ICC	IRC-18	International Residential Code	Table B105.1(1)
NFPA	1142-17	Standards on Water supplies for Suburban and Rural Fire Fighting	B104.2

4.8.3 Hydrant Locations

- A. The spacing of fire hydrants is to be measured along the street or roadway in which a fire hose would be laid. This spacing is measured along the curb line and shall be inclusive of the distance along a private driveway to the proposed structure.
- B. The Fire Department will stipulate fire hydrant locations during the site planning process or on the final plans review. The following standards shall be used as a guide:
 1. The Fire Department will stipulate fire hydrant final locations during the site planning process or during the plan review
 2. Fire hydrants shall be provided along required fire apparatus access roads and adjacent public streets
 3. Fire hydrants shall be set every four hundred (400) feet or closer as determined by the Fire Marshal. The minimum number of fire hydrants available to a building complex or subdivision shall not be less than that determined by spacing requirements listed in the adopted Fire Code or as directed by the Fire Marshal.
 4. Existing fire hydrants on public streets may be considered for adjacent development. Existing fire hydrants on adjacent properties shall not be considered available unless fire apparatus access roads extend between properties and easements are established to prevent obstruction of such roads

5. Regardless of the average spacing, fire hydrants shall be located such that all points on streets and access roads adjacent to a building are within the distances listed in the adopted Fire Code

4.8.4 Fire Lines & Building Sprinkler Lines

- A. The location of on-site fire lines and taps shall be determined by the relative location of the fire department connection, riser location, emergency access and fire hydrant locations.
- B. The size of fire lines shall be determined by the Engineer of Record for design of the project based on required fire flow demands. Fire systems shall include a backflow preventer in accordance with Town of Camp Verde Standards.
- C. Show all fire lines on the approved civil site plan.

4.8.5 Fire Department Connection

- A. If a remote Fire Department connection for a sprinkler system is required, it shall be installed thirty (30") inches from the back of curb, on-site driveway, or sidewalk. The FDC should be thirty-six (36") inches in height. The location of the sprinkler system connection shall be unobstructed and readily accessible to the Fire Department. This connection shall also be within an appropriate distance of a fire hydrant as determined by the Fire Department.
- B. Fire Department connections, whether remote or wall mounted, need to be identified and coordinated on the improvement plans and on the building plans site plan for relationship to fire lanes and fire hydrants. All Fire Department connections shall be appropriately clear of glazing and other hazards and protected from vehicular damage.
- C. Pavement markers for Fire Department sprinkler system connections shall be provided.

4.8.6 Auxiliary Storage Tanks

Minimum water pressures and discharge flow shall be in accordance with the adopted Fire Code. A fire pump package installation may be required when the building's construction type, occupancy fire load commodities classification, volumetric building areas, building height and individual square footage areas per floor level produce a pressurized fire flow supply in excess of the water transmission mains capabilities.

5 RECLAIMED WATER

5.1 INTRODUCTION

5.1.1 Purpose

The goal of the Reclaimed Water System Planning and Design Section is to provide technical management, comprehensive planning, and sound engineering to expand and maintain a reliable and safe reclaimed water supply, distribution, and storage system.

5.1.2 Applicability

- A. The standards and guidelines of this Section shall apply to the construction, modification and extension of all reclaimed water distribution mains, pump stations and other appurtenances as well as any modification of the public reclaimed water system to be owned, operated and maintained, by the Town of Camp Verde (TOCV).

5.2 GENERAL INFORMATION

5.2.1 Reclaimed Water Availability

The Town of Camp Verde reclaimed water is currently used to irrigate the Sports Complex grass fields and trees. The Town is working to bring reclaimed water to Main Street to irrigate landscaping and the grass field on East Hollamon in the future. Reclaimed water is available for construction and dust control - Call (928) 554-0850 or email utilities@campverde.az.gov.

6 WASTEWATER

6.1 INTRODUCTION

6.1.1 Purpose

The purpose of this Section is to provide engineering standards, guidelines, and geometric requirements for the design and construction of the public wastewater collection system and private sewer systems that discharge into the public collection system within the Town of Camp Verde.

6.1.2 Applicability

- A. The standards and guidelines of this Section shall apply to the construction and modification of the public wastewater collection system and any private sewer system that discharges into the public collection system. This Section is intended for use in plan design, plan preparation, and the plan review process. The information provided in this Section is not intended to cover all situations that arise, nor may it be a substitute for sound engineering principles and judgment.
- B. Developers are required to install all improvements necessary to provide wastewater service to their development. This includes any sanitary sewer main, lift station, force main, manhole, vault or other facilities, and required appurtenances, including the payment of all applicable development fees.
- C. Developers shall also adhere to the Town of Camp Verde's standards for extension of the Town of Camp Verde's wastewater system to newly developed areas, subdivisions and in-fill areas inside the Town of Camp Verde's service area in accordance with the Town of Camp Verde's Wastewater Collection Model.

6.1.3 Design Standards & Guidelines

- A. All public sanitary sewer systems are to be of a gravity flow design, unless other factors dictate the use of a force main and lift station. For lift station and force main requirements, refer to EDSM, Section 6.7.2 (Wastewater Lift Station) and EDSM, Section 6.7.3 (Force Main). Developments that need to construct lift station facilities and force mains shall address the facilities compatibility with the Town of Camp Verde's Wastewater Collection Model.
- B. New public wastewater improvements shall be designed in accordance with Section 1.
- C. A civil engineer registered in the State of Arizona shall analyze, design and seal all proposed wastewater system project plans, reports and specifications that are determined by the Town Engineer to have an impact on the wastewater collection system. The effects of peak flow shall be examined to ensure proper sizing and

layout of proposed wastewater system facilities, including impacts to existing offsite facilities.

6.1.4 Federal, State & County Regulations

A. Environmental Protection Agency (EPA) Regulations

1. The U.S. Environmental Protection Agency (EPA) requires the Town of Camp Verde to develop and implement a program to control discharges that might harm the Publicly Owned Treatment Works (POTW). The program establishes local discharge limits for non-residential users, and provides a permitting process based on the users' discharges and types of businesses.
2. The Environmental Protection Agency (EPA) wastewater flow report (EPA 625/R-00/0008 – Chapter 3), states “great care shall be exercised in predicting wastewater characteristics so as not to accumulate multiple factors of safety that would yield unreasonably high design flows...”. Further, “...actual measurement of wastewater flows and quality from a residential dwelling or non-residential establishment always provides the most accurate estimate for sizing and designing...” Therefore, the priority of obtaining water and wastewater design flows is 1) flow measurements from the actual facility, 2) the use of flows from similar facilities in the immediate area, 3) the use of similar flows from similar facilities in other areas and, 4) flow tables.

B. Arizona Department of Environmental Quality (ADEQ) Engineering Bulletin No. 11, "Minimum Requirements for Design, Submission of Plans and Specifications of Sewage Works" published by the Arizona Department of Environmental Quality (ADEQ), and Arizona Administrative Code (AAC), "Title18 - Environmental Quality" (R18-9), contain specific requirements for submittals, approvals, and notifications when extension of a public sanitary sewer main is proposed. Additional information can be found on the ADEQ website at www.adeq.gov/function/permits/index.html.

1. Before a permit is issued for the construction of civil infrastructure, the developer shall submit to the Town Engineer, Construction Authorization from ADEQ.
2. Before the Town Engineer will issue an Approval to Operate (ATO) and Release from the Town of Camp Verde, the developer shall deliver to the Town Engineer, a signed Discharge Authorization from ADEQ.

C. Yavapai County Environmental Services Department (YCESD)

1. Before a permit is issued for the construction of civil infrastructure, the developer shall submit to the Town Engineer, Construction Authorization from ADEQ through the YCESD delegation.
2. Before the Town Engineer will issue an Approval to Operate (ATO) and release from the Town of Camp Verde, the developer shall deliver to the Town Engineer, a signed Discharge Authorization from ADEQ through the YCESD delegation.

D. Town of Camp Verde (TOCV)

1. Issuance of an Approval to Construct by YCESD or ADEQ does not constitute an approval of plans and reports by the Town of Camp Verde. No construction shall commence prior to the Town of Camp Verde Town Engineer issuing a permit.
2. An As-Built submittal is required prior to receiving an Approval to Operate and Release from the Town of Camp Verde.

6.2 GENERAL INFORMATION

6.2.1 Availability of Public Town Sewer

- A. The Engineer of Record shall submit the required forms for ADEQ approval to the Public Works Department.
 1. ADEQ approval is delegated to Yavapai County Environmental Services. Refer to Yavapai County website www.azdeq.gov for further information and applicable forms.
- B. Average daily design flows shall be used for the Treatment Capacity Assurance, while appropriate peaking factors shall be applied to flows for the Collection Capacity Assurance.
- C. In order to prevent the possibility of differing ADEQ and Town of Camp Verde approved plans, the Town of Camp Verde will not sign and release the Capacity Assurance Forms until all design and review comments have been resolved. Design and construction scheduling shall account for this requirement. Design issues relative to capacity shall be resolved prior to forms being signed.

6.2.2 Private Wastewater Areas

- A. Private wastewater collection providers serve portions of Camp Verde's municipal service area. Placing private sanitary sewer mains within Town of Camp Verde rights of way (ROW) shall require a license agreement between the private wastewater company (e.g. LLC or HOA) and the Town of Camp Verde.
- B. The private wastewater service provider shall review modifications or construction of wastewater systems within their franchise areas. When submitting final plans, the developer shall provide written documentation that the private wastewater service provider has reviewed and approved the sanitary sewer mains within its jurisdiction.
- C. The Town of Camp Verde shall review all private wastewater systems as part of the plan review process. A note shall be placed on the recorded plat or lot split survey and on the construction drawings stating the sewer collection system is private or public as well as operation and maintenance responsibilities on each specific portion of the system constructed. The plan cover sheet shall include a signature block for responsible party for maintenance.

- D. Private wastewater system connections to the Town system shall be built to Town of Camp Verde standards for public sewer force main discharge as identified in EDSM, Section 6.7.3. Required testing of the system shall be performed after backfill and compaction have been completed.

6.2.3 Private On-Site Systems / Alternative Treatment Facilities

The Town of Camp Verde does not issue approval for the installation of on-site treatment and disposal systems (i.e. septic or alternative systems). Approval for on-site systems, in lieu of connection to the Town of Camp Verde public sanitary sewer system is reviewed and permitted by Yavapai County Environmental Services Department under Delegation Agreement from ADEQ. The Town will also review the site plan. The property owner is responsible for the design, permitting, construction, operation, and maintenance of these systems in accordance with all permit conditions.

6.2.4 Private Gravity Mains / Low Pressure Sewer Systems

- A. The Town of Camp Verde does not own, operate or maintain Low Pressure Sewer (LPS) Systems as these systems are considered private by the Town of Camp Verde. For connection of an LPS to the Town of Camp Verde system for more than one home, the language from 6.2.4.B.1 is to be added to the project plans, plat, CCR's and the deeds. Formation of a Limited Liability Company (LLC), or other entity approved by the Town of Camp Verde Legal Department for perpetual maintenance responsibilities shall be required.
- B. Operation, Maintenance and Repair of Common Elements and Units
 - 1. Operation, Maintenance and Repair of the Private Sewer System. The individual sewer customers are served by private sewer collection systems, located as shown on the final as-built project plans, the operation and maintenance of which are the responsibility of the Wastewater Service Provider (Provider) in perpetuity. The Provider shall pay the cost of all operation, maintenance, repair and replacement, of private systems. These private systems are connected to main lines operated by the Town of Camp Verde, as identified on the final approved plans. The Town of Camp Verde shall have no obligation or requirement to assist or provide manpower or equipment to perform or supplement any responsibilities pertaining to said private sewer systems, all of which are those of the Provider. The Provider shall provide on a yearly basis the firm, contact name, business phone number and emergency phone number for the private system operator, who shall be certified by the Arizona Department of Environmental Quality at the required level.
- C. Acceptance of Private Sewer Systems for Town of Camp Verde Ownership, Operation, and Maintenance. The legal entity shall not petition the Town of Camp Verde to accept the private system or any portion thereof for ownership, operation

or maintenance until the systems are reconstructed to meet Town of Camp Verde standards in effect at the time of said petition.

6.3 PLAN SUBMITTAL REQUIREMENTS

- A. The improvement plans submitted to the Town of Camp Verde for approval shall adhere to professional engineering standards governed by the State of Arizona Board of Technical Registration. Improvement plans shall be complete and shall comply with Town of Camp Verde and State Board requirements. Improvement plans that do not meet industry standards, or are incomplete, shall be returned by the Town of Camp Verde without review comments and shall be determined to be an inadequate project submittal. Additional plan review fees may be required.
- B. A civil engineer registered in the State of Arizona is required to analyze the wastewater generation from a proposed development while determining the closest and best connection point through discussion with the Town Engineer. The Engineer of Record shall also determine its impact on the Town of Camp Verde's wastewater collection system. This analysis is required from the development to a point on the downstream system where the engineer can certify compliance with a model previously accepted by the Town of Camp Verde. The Town of Camp Verde is responsible for analysis of sanitary sewer mains shown in the Town of Camp Verde's most recently adopted Water and Wastewater Models.

6.4 WASTEWATER SYSTEM REPORTS

6.4.1 General Information

- A. The developments wastewater master plan and design reports provide an analysis of the impact that development will have on the Town of Camp Verde's wastewater collection system. These reports are reviewed and accepted by the Town Engineer.
- B. Analysis of all proposed development determined by the Town of Camp Verde to have an impact on the wastewater system shall be performed by a civil engineer, registered in the State of Arizona. The analysis shall include the effects of peak flow to ensure proper sizing and layout of the proposed wastewater system facilities.
- C. The objectives of the Wastewater Reports are to determine the development's wastewater generation rate, analyze the hydraulics of the proposed sanitary sewer system to a point of connection within the Town of Camp Verde's Wastewater System Model and demonstrate conformance for each phase of the development with the accepted master plan for that development. All reports submitted to the Town of Camp Verde for review shall be prepared in accordance with the following guidelines:
 - 1. Format:
 - a. The report shall be submitted in pdf format
 - b. All reports shall have a table of contents

- c. A civil engineer licensed to practice in the State of Arizona shall seal each report
 - d. The project name shall be located on the cover
 - e. The name, address, and phone number of the developer/owner and engineer shall be on the cover
 - f. The original submittal date and all subsequent revision dates shall be located on the cover
 - g. Common spreadsheet formats shall be compatible with MS Excel
2. Existing Conditions:
- a. Provide a site description, size, addresses, major streets, township, range and section, relationship to other developments or significant sewer features
 - b. State the existing zoning and land use
 - c. Describe the existing, topography, and landform features
 - d. Describe the existing wastewater collection system infrastructure affected by the development. Identify lift stations and downstream trunk mains that will be used to convey wastewater from the development
 - e. Reference any existing master plan(s) or design report(s), applicable from adjacent development. Include excerpts from existing plans or reports, as applicable
3. Proposed Conditions:
- a. Summarize the proposed development, including project name, size and type of development. Include a site plan that indicates the layout of the proposed development with a survey one hundred (100') feet beyond the project limits
 - b. Describe infrastructure required to convey wastewater from the development to the wastewater treatment plant. Show all proposed connection(s) to the Town of Camp Verde's wastewater system. Show extension of sanitary sewer mains into and within the site
 - c. Summarize the ADEQ and/or Town of Camp Verde's design standard requirements that were applied to this project
 - d. Address ownership, operation and maintenance responsibilities of the proposed wastewater system
 - e. Design Methodology
 - i. Sanitary sewer main design shall provide gravity collection and buildout flows from the ultimate service area and shall allow for future extension of service to adjacent parcels when possible
 - ii. Modeling – Identify the software used and specific assumptions in the model

- iii. Wastewater Flows – Identify land use, population, population density, loads and peaking factors
 - iv. Collection System Network – Identify and describe mains that will be required to serve the development
 - v. Lift Stations
 - 1. Describe temporary and/or permanent lift stations where required
 - 2. Provide design parameters, and layouts when appropriate
 - 3. Lift stations shall be designed in accordance with Town standards for materials, equipment, communication and backup power
 - vi. Outfalls – Identify and describe location and size
 - f. Wastewater Model and Results
 - i. Identify main sizes, slopes, and other results as obtained from the model
 - ii. Include a map of the collection system for each simulation where pipes are color coded or appropriately labeled
 - iii. Any interim areas that do not have sufficient self-cleansing velocity and may require additional maintenance activities until build out shall be clearly identified
 - g. Table summarizing parcels, acreages, land use, and population
4. Mapping & Exhibits:
- a. All mapping shall conform to Town of Camp Verde standards and the EDSM, Section 9
 - b. The project name shall be located on each sheet
 - c. The name, address, and phone number of the Engineer of Record shall be stated on each sheet
 - d. The original submittal date and all subsequent revision dates shall be located on the cover
 - e. All maps shall be to scale
 - f. Include a vicinity map identifying the project's location with respect to major cross streets
 - g. All proposed or existing on-site and off-site facilities, including, but not limited to, wastewater mains, lift stations, manholes, etc.
 - h. All proposed or existing street locations, parcel boundaries, and proposed lots within each parcel. Show dimension and label clearly all property lines, rights-of-way, tracts and easement lines
 - i. Contour lines, shall have a maximum of two (2') foot intervals and ten (10') foot labels

- j. Graphics shall present existing utilities as dashed lines and proposed utilities as bold solid lines
- k. Screen existing topography into the background. Clearly label existing and proposed contour lines at ten (10') foot intervals. Show sufficient information to evaluate pipe cover.

6.4.2 Wastewater Master Plan Report Content

- A. A wastewater master plan is required when phased construction is proposed or if required by the Town Engineer. The Wastewater System Master plan shall show compliance with the Town of Camp Verde's design criteria and development policies for each phase of the project and establish a skeletal system for the phased development of a master planned project.
- B. The Wastewater Master Plan Report shall specify the terms and requirements for wastewater service to the development.
- C. All development projects shall be responsible for determining their specific wastewater discharge and shall include buildout flows from any upstream developments to ensure the system is designed properly.
- D. If the proposed development contemplates a change in the original master plan or zoning, which increases density or increases wastewater system flows different from the Town of Camp Verde's existing Wastewater System Master Plan, then additional downstream analysis shall be required.
- E. Flows shall be calculated according to the hierarchy established in the EDSM, Section 6.1.4.A.2.
- F. Along with the model report, a USB flash drive containing all analysis and modeling shall be submitted along with the model report if the project is determined to be of sufficient complexity and compatible with EPA-NET model software.
- G. Each Model map shall show the following:
 - 1. All proposed on-site and off-site facilities including, but not limited to, lift stations, trunk mains and service connections and laterals
 - 2. Proposed street locations, parcel boundaries and proposed lots within each parcel
 - 3. Contour lines, with a maximum of two (2') foot intervals
 - 4. A separate area location map showing existing and proposed streets, as well as existing parcels surrounding the project to a distance of one (1) mile from the exterior boundaries of the project
 - 5. A scale that is sufficient to show all required information clearly
- H. The Town of Camp Verde requires wastewater mains to be installed along the entire length of the property line frontage of the property being developed. The property line frontage is defined as that portion of a parcel of property that abuts a street, easement, or public rights of way. If a parcel to be developed has more than one (1)

frontage, improvements shall be installed along all frontages where future connectivity is possible.

- I. A construction phase matrix shall be included in a table format for all wastewater related infrastructure required to serve the development. The table shall include each phase, parcel, and each system component. A mark in each box shall specify when infrastructure is anticipated to be required, for each phase of the development.
- J. The master plan report shall comply with the adopted Town of Camp Verde Wastewater System Master Plan encompassing the respective area.

6.4.3 Wastewater Design Report Content

- A. All development projects shall be required to provide an analysis of the project's impact on the Town of Camp Verde wastewater system. If the project involves a subdivision plat or lot split a preliminary design report is required with submission of the preliminary plat or lot split survey. The final sewer system design report shall be submitted with the civil plans. The civil plans shall not be approved until the sewer system design report has been approved.
- B. The report shall determine the development's wastewater hydraulic load and analyze the hydraulics of the proposed sanitary sewer system and its impact to the Town's existing system. Additionally, commercial and industrial developments shall include both hydraulic and concentration loads. The analysis shall include the effects of the peak flow to ensure correct sizing and layout of the new wastewater facilities.
- C. Design Documentation
 - 1. Summarize the proposed development. Include a description based on aliquot parts or reference within a platted development and describe the proposed uses
 - 2. Include a narrative of which design procedures, policies and methodologies will be incorporated into the design of the proposed wastewater system
 - 3. Note compliance with the EDSM and all other applicable standards and codes on the design report
 - 4. List the title and version of any software used in the design analysis
- D. Analysis
 - 1. Base wastewater flows in accordance with EDSM, Section 6.4.1.A.2
 - 2. Verify any variance from the stated design flows with the Utilities Department
 - 3. Identify wastewater peaking factors and peak flows
 - 4. Scour analysis if drainage crossings are required
 - 5. Gravity Main
 - 6. Force Main
 - 7. Lift Station
 - 8. Appurtenances

9. Summary Tables

- E. Wastewater modeling results with exhibits shall be included.

6.4.4 Sewer System Model Analysis

- A. Provide a model of the planned wastewater system as delineated in this section of this manual. This modeling shall be required where new wastewater infrastructure is being added to the existing Town of Camp Verde system. The model shall be provided in a format compatible with the Town of Camp Verde's master model and verify flows in the Town of Camp Verde system. Minor infill projects may not require a model at the discretion of the Town Engineer.

1. Wastewater Model Requirements:

The developer or developer's engineer shall create and utilize a hydraulic model to demonstrate that the proposed infrastructure of the wastewater system is adequate and satisfies the performance criteria. Hydraulic modeling information shall be provided to the Town Engineer.

2. Hydraulic Model Data:

- a. All final drawings and model data to be submitted to the Town shall use the Town of Camp Verde EDSM, Section 9 for coordinate system information.
- b. Attributes:
 - i. Each physical entity in the model of the proposed development shall require information and attributes as defined below
 - ii. Wastewater Collection System
 - iii. Gravity Mains – Diameter, material, length, Manning's coefficient, location, connectivity with mains and manholes, upstream invert elevation, and downstream invert elevation
 - iv. Manholes – Diameter, location, manhole losses (minimum one tenth (0.1) foot) drop connecting mains, drop manhole details, wastewater load, and invert elevations
 - v. Lift Stations – Layout, location, and connectivity with other entities, number of pumps, pump curve, pump start/stop controls, rim elevation, wet well dimensions, wet well invert, outfall location and elevation
 - vi. Force Mains – Diameter, material, length, Manning's coefficient, location, connectivity and manholes, upstream invert elevation, and downstream invert elevation
 - vii. Diversion Structures – Diameter, invert elevation, location, connectivity, weir height and size, control scheme, desired flow through each diversion
 - viii. Outfalls – Location, connectivity, invert elevation

3. Hydraulic Evaluation:

New developments shall be modeled to determine if the proposed infrastructure is adequate to service the development and provide the level of service as defined by the Town of Camp Verde's performance criteria.

- a. The Town Engineer will use the Town of Camp Verde's model to determine the available capacity in the Town's existing downstream sewer collection system to convey wastewater from the development per the design engineer's proposal. The developer will model proposed collector mains and will be responsible to make certain that wastewater can be safely conveyed from the points of origin to the Town of Camp Verde's trunk mains. Off-site improvements to the collection system and lift stations may be required for safe conveyance.
- b. Boundary conditions that represent the interface between the development and the rest of the collection system need to be identified and any deficiencies made clear.
- c. All improvements required to provide sufficient capacity for project flows shall be identified.
- d. Steady-state simulations shall be completed utilizing dry weather average day flow multiplied by the appropriate upstream population peaking factor plus ten (10%) percent Inflow and Infiltration (I&I) factor for new construction. Design documentation shall show that the collection system mains are sized adequately and that slopes are greater than the minimum slope to maintain self-cleansing velocities.

FIGURE 5: SEWER COLLECTION DESIGN CRITERIA

CHARACTERISTIC	PARAMETER
Gravity Mains	$2 \text{ fps} \leq v \leq 10 \text{ fps}$
Force Mains	$3 \text{ fps} \leq v \leq 7 \text{ fps}$
Flow Depth ,d/D	
d/D for New Sewer Mains with Diameters less than 12-inches	= 0.50
d/D for New Sewer Mains with Diameters greater than or equal to 12-inches	= 0.75
d/D for Evaluating Existing Mains in Developed Areas	= 0.90
Head Loss in Existing Mains	
Gravity Mains	Manning's N = 0.013
Force Mains	Hazen William's C – 120 max
Changes in Main Size	
When a Smaller Main joins a Larger Main	Match Crowns
Head Loss at Manholes	
Manholes with Mains Intersecting at 90° or Greater	Provide a Minimum of 0.1' Invert Drop

6.5 WASTEWATER DESIGN FLOWS

6.5.1 Wastewater Collection Analysis

This section shall describe the measurement standards for gravity sewer mains that are to be used to evaluate the suitability of wastewater collection system improvements and additions.

6.5.2 Wastewater System Loads

All projects shall provide a summary of the anticipated sewer loads for the project. Provide an analysis of the anticipated sewer flows.

6.5.3 Peaking Factor

- A. The peaking factor shall be as defined in the Arizona Administrative Code Section R18-9-E301.D.1.b.
- B. In addition to dry weather peaking factors, ten (10%) percent shall be added for wet weather inflow and infiltration (I&I).

- C. Where population estimates are not available or non-residential loads prevail, dry weather peaking factors can be calculated using:

$$PF = C(Q_{avg})^{-M}$$

Where:

C = constant (empirical coefficient)

Q_{avg} = average flow rate (ft³/s, m³/s)

M = exponent

6.6 INDIVIDUAL WASTEWATER TREATMENT FACILITIES

6.6.1 On-Site Treatment & Disposal Facilities

- A. When wastewater service is not available from the Town of Camp Verde system, an on-site treatment and disposal system (septic or alternative system) is required. Permitting is required through the Yavapai County Environmental Services Department (YCESD) for all on-site treatment.
- B. The property owner is responsible for the design, construction, operation and maintenance of septic systems/on-site wastewater treatment and disposal facilities. The Town of Camp Verde shall not accept any type of on-site system for operation and maintenance but may provide input to the YCESD regarding environmental concerns.
- C. All on-site wastewater treatment and disposal facilities shall be designed, constructed and maintained by the property owner compliant with the applicable requirements of YCESD.

6.6.2 Wastewater Lift Stations

- A. When lift stations are contemplated, the Engineer of Record shall meet with the Town Engineer to discuss development options and design requirements.
 - 1. Site selection for wastewater lift stations shall consider accessibility, drainage patterns, visual and neighborhood impact, three-phase power availability, function and design constraints.
 - 2. The potential for flooding shall be considered when selecting a wastewater lift station location. The station's equipment, vaults and manholes shall be protected from damage and remain operable during a one hundred (100) year flood.
 - 3. Each tract or lot used for lift station facilities shall include all weather ingress/egress.

4. Wastewater facilities shall be located on a tract or lot dedicated to the Town of Camp Verde (conveyed by a general warranty deed) and accompanied by a title policy accepted by the Town of Camp Verde.

B. Lift Station Requirements

1. Arizona Administrative Code, Title 18, Chapter 9, “Water Pollution Control,” and ADEQ, Bulletin 11 contain minimum requirements for a wastewater lift station. Additional requirements specific to the Town of Camp Verde (including but not limited to wet well coating, SCADA, Electrical, pump preferences, etc.) shall be obtained from the Town Engineer before beginning design. At a minimum telemetry, pump and wet well redundancy, backup power supply, three-phase power, odor control and security perimeter walls shall be required. The site shall be large enough to fully contain all the facilities and service equipment for repairs and maintenance.
2. A preliminary design report shall be prepared and submitted to the Town Engineer for review with the preliminary plat. The preliminary report shall outline the type of equipment and controls proposed for the station that meets the Town of Camp Verde equipment standards.
3. A final design report prepared by a registered professional engineer, licensed in the State of Arizona, shall accompany all lift station design drawings and specifications submitted to the Town of Camp Verde for review with the final plat.

C. Lift Station Design

1. In addition to State requirements lift stations shall be sized per the following:
 - a. The wet well shall be sized so that the number of duty pump starts per hour does not exceed three (3) during max day flows, including Inflow & Infiltration (I&I)
2. Design for current, interim and ultimate conditions.
3. Station shall have a minimum of two (2) pumps and be capable of operating at the designed flow with any one pump out of service.
4. The individual pump capabilities shall be identical to facilitate O&M.
5. Pumps shall be submersible or flooded suction dry pit and shall pass a minimum three (3”) inch sphere or are grinder pumps.
6. Include valve vault with shut-off and check valves.
7. Aeration and odor control:
 - a. The Odor Control system (biofilter, biofilter with optional secondary activated carbon polishing unit, or stand-alone activated carbon filter unit will be sized commensurate with hydrogen sulfide concentrations, wet well
 - b. The unit must be in a non-corrosive container with a four (4”) inch minimum static air vent or a VFD controlled centrifugal fan installed

- with a sound attenuation enclosure, as determined by the sizing requirements
- c. Aeration of the wet well shall be required where the retention time exceeds thirty (30) minutes
 - d. A compressor with diffuser bar shall be placed in the wet well, with the compressor sized for two (2) standard cubic feet per minute (SCFM) per one thousand (1,000) gallons of wet well storage
 - e. The diffuser piping shall be constructed of stainless steel with coarse bubble diffusion and deflector shield
- 8. Lockable aluminum access covers shall be located directly over the pumps and shall be a minimum of twenty-four (24") inches clear of any equipment in order to remove and replace pumps. Aluminum hatches shall include non-corrosive hardware and internal safety netting.
 - 9. Electrical control unit (EG Controls) shall not be located on the wet well or adjacent manholes and shall have separate mounting pad and enclosure.
 - 10. Pre-cast or cast-in-place concrete walls shall be protected against corrosion. All interior walls shall be lined using Sewer Shield 150, Sewer Guard 210S or Raven 405 and installed by a certified installer in accordance with manufacturers specifications.
 - 11. Controls shall have hour meters, cycle meters, magnetic flow meter and run and failure lights with audible alarm and rotating beacon light, hand off auto switches, and provide for alternating sequencing of pumps.
 - 12. All pump rails and hardware shall be stainless steel.
 - 13. A diversion by-pass line and manhole with diversion piping and valves shall be included for routine maintenance and emergency pumping.
 - 14. An emergency power source shall be provided that will supply immediate service and have an automatic transfer switch.
 - 15. Communication to Town of Camp Verde's SCADA monitoring system, for operations, monitoring, and security.
 - 16. Equipped with visual or audible alarms when exceedingly high-water levels are detected.
 - 17. Level controls shall be avocado style floats, normally open.

6.6.3 Force Mains

- A. Force mains shall be located within a public right-of-way, or a utility easement. The main shall be located under pavement where possible.
- B. The flow velocity in the force main shall be between three (3') feet and seven (7') feet per second (fps). Force main detention times and the potential for hydrogen sulfide generation shall be calculated for any interim phasing and full project buildout. Parallel mains of differing sizes may be required if interim/startup conditions result in insufficient discharge rates, velocities and excessive retention times.

- C. All pipe material used in design of the force mains shall have established ASTM, ANSI, AWWA and NSF standards of manufacture or seals of approval. All pipes shall be designated as pressure sanitary sewer pipe. Acceptable materials shall include epoxy lined Ductile Iron Pipe (DIP) or Polyvinyl Chloride (PVC).
- D. Air/vacuum valve(s) and all appurtenances designed for wastewater shall be stainless steel. Air/vacuum valve(s) shall be provided on force mains at all local high points. Refer to TOCV Standard Detail 416CV.
- E. Cleanouts and flushing points may be required based on length, flow, and topography.
- F. Refer to MAG Standard Detail 426 for details regarding discharge into a manhole from a force main.
- G. The separation between the force mains and water and reclaimed water mains shall be six (6') feet exterior wall-to-wall vertically and six (6') feet horizontally. Where a force main crosses above, or less than six (6') feet below a water or reclaimed water main, as identified in MAG 404-1 and MAG 404-2, both the water or reclaimed water main and sewer main shall have additional protection per AAC Title 18, Chapter 5, Section 502(C).
- H. Odor control and epoxy lining shall be required at the receiving manhole and shall be sized according to the air volume and pumping. Additional odor control and epoxy lining may be required on downstream facilities. Acceptable means of odor control include bio-filtration, filtration by media such as activated carbon, Chlorine Dioxide, Permanganate, Alumina or Sulfatreat or other method approved by the Town Engineer.
- I. A tracer wire shall be attached to the force main as part of the installation per TOCV Standard Detail 319CV

6.6.4 Flow Metering Structure

- A. The Public Works Department may require the installation of a flow metering structure. When required, the engineer shall design the flow metering structure as part of the project's improvements. The flow metering structure design shall include:
 - 1. Adequate land area for the structure with access to public ROW
 - 2. Security perimeter fence for site
 - 3. Locking structure for appurtenances
 - 4. SCADA radio monitoring system
 - 5. Electrical control system, to include software
 - 6. Flow measurement and recording (magnetic or ultrasonic type)
 - 7. Pipe and conduit penetrations in flow metering structure shall be core drilled. Pipe penetrations shall be sealed with Link Seal
 - 8. Aluminum access hatches with minimum four (4') feet by six (6') feet clear opening

9. Fiberglass reinforced polyester grating
10. All vault interior walls shall be lined using Sewer Shield 150, Sewer Guard 210S or Raven 405 and installed by a certified installer in accordance with manufacturers' specifications

6.7 COLLECTION SYSTEM

6.7.1 General Requirements

- A. Property shall be abutting a sewer main for a service connection to be made, unless otherwise allowed pursuant to this section. In cases where the main is not abutting, the main shall be extended at the expense of the property owner before a connection is made.
- B. The Town of Camp Verde requires sewer mains to be installed along the entire length of the property line frontage of that property being developed, unless constrained by topography. The property line frontage is that portion of the property that abuts a street, public utility easement or public ROW. If a parcel to be developed has more than one (1) property line frontage the Town of Camp Verde may require a sanitary sewer main be installed along the entire length of all frontages where parcel development paving is required, and future connectivity is possible.
- C. Developers shall install, at their expense, all on-site and off-site sewer improvements necessary to serve their development, including but not limited to gravity mains, force mains, lift stations, and any improvements to existing Town facilities to obtain all permits and meet regulatory compliance.
- D. All building sanitary sewer service lines shall be privately owned, operated and maintained.
- E. Sewer mains installed by improvement projects, which are adjacent to undeveloped parcels, are required to install sewer main stubs from the adjoining manhole(s) to facilitate the future extension of the public sewer system to serve future developments. Stubs from the manholes are to be extended to the ROW line and be eight (8") inches minimum in diameter and finished with a vertical clean-out riser and MAG Standard Detail 270 monument.
- F. When required by the Town Engineer, users who discharge non-residential wastewater shall install monitoring manholes.
- G. Any sewer mains not built to Town Standards, shall be privately owned, operated, and maintained. Refer to EDSM 6.2 for additional requirements.

6.7.2 Design Policy

- A. Reconstruction of residential or commercial structures requires compliance with all current ordinances and design guidelines relating to sewer main extension.

- B. For developments where an existing sanitary sewer is not available, a dry sanitary sewer main, shall be installed and tested in conformance to all the design requirements, for a public sanitary sewer main. Design a temporary on-site wastewater treatment system, located such that future connection is possible to the dry system when sanitary sewer service becomes available.
- C. Wastewater systems shall be designed to serve the ultimate population density expected in the collection basin. The design shall be in conformance with the current Town of Camp Verde approved wastewater system model and take into consideration future connections. Where a wastewater collection system extension is possible upstream of a subdivision, extend the sanitary sewer through the subdivision to the platted boundary to a point of connection that shall provide wastewater service to adjacent properties. Regulatory guidelines shall be met in order to provide adequate capacity for upstream flows without future reconstruction.

6.7.3 Design Standards

Wastewater collection systems shall be designed in compliance with the Arizona Administrative Code, Title 18, Chapter 9, “Water Pollution Control”, Arizona Department of Environmental Quality (ADEQ), Bulletin 11, and the Town of Camp Verde EDSM.

6.8 MATERIALS

6.8.1 Pipe Selection

In selecting pipe material for sanitary sewer mains, consideration shall be given to the chemical characteristics of wastewater, especially industrial waste. Consider velocity, the possibility of septicity, external and internal pipeline forces and preventing infiltration, abrasion, and similar type problems.

6.8.2 Materials

- A. The following types of sanitary sewer mains and force mains may be used:
 - 1. Polyvinyl Chloride (PVC) SDR26, up to fifteen (15”) inches in diameter
 - 2. PVC (meeting ASTM F679-16) between eighteen (18”) and twenty-seven (27”) inches in diameter; or
 - 3. Class 350 (CL350) Ductile Iron Pipe (DIP) epoxy coated, and seal coated.
- B. Pipe materials may not change between manholes, unless extra protection is required for utility separation.
- C. Trench loading conditions shall be analyzed for each pipe type used. The pipe used, including bedding requirements, shall meet Town and manufacturers specifications for each condition.
- D. Pipe material used in design shall have established ASTM, ANSI or NSF standards of manufacture or seals of approval and shall be designated for use with wastewater.

6.8.3 System Layout

- A. Public sanitary sewer mains within commercial, industrial or multi-family developments shall be located within drive aisles and a minimum of ten (10') feet from any structure, including overhead projections. The main shall also be located a minimum of five (5') feet from any parking stall. Public sanitary sewer mains shall be located within Town of Camp Verde ROW or public utility easements. No private services are allowed longitudinally within a public utility easement or Town of Camp Verde ROW.
- B. If the horizontal direction, slope, material or size of the sanitary sewer main changes, a manhole shall be constructed. The horizontal angle formed between an inlet and outlet pipe shall not be less than ninety (90°) degrees.
 - 1. Curvilinear sanitary sewer mains shall not be allowed.

6.8.4 Design Flows

- A. The priority of obtaining wastewater design flows is 1) flow measurements from the actual facility, 2) the use of flows from similar facilities in the immediate area, 3) the use of similar flows from similar facilities in other areas and, 4) flow tables.
- B. Peaking factors in accordance with Table 6-2 shall be used, plus ten (10%) percent inflow and infiltration (I & I).

FIGURE 6: WASTEWATER FLOW PEAKING FACTOR

UPSTREAM POPULATION	DRY WEATHER PEAKING FACTOR
100	3.62
200	3.14
300	2.90
400	2.74
500	2.64
600	2.56
700	2.50
800	2.46
900	2.42
1,000	2.38
1,001 to 10,000	$PF = (6.330 \times p^{-0.231}) + 1.094$
10,001 to 100,000	$PF = (6.177 \times p^{-0.233}) + 1.128$
More than 100,000	$PF = (4.500 \times p^{-0.174}) + 1.945$
PF = Dry Weather Peaking Factor p = Upstream Population	

6.8.5 Hydraulic Design

- A. No public sanitary sewer mains shall be less than eight (8") inches in diameter.
- B. Sanitary sewer mains shall be designed and constructed to provide full flow velocities of not less than two and one-half (2.5) feet per second (fps), based upon Manning's Formula, using an "n" value of thirteen thousandths (0.013).
- C. The design engineer shall analyze all mains at average day and peak flow and provide velocities as close to two (2) fps as possible (average day). Any velocities under two (2) fps (average day) shall be noted as requiring additional maintenance.
- D. To prevent abrasion and erosion of the pipe material, the maximum velocity shall be below ten (10) fps at estimated peak flow. Where velocities exceed ten (10) feet per second (fps), the main shall be constructed of a pipe material that is abrasion resistant. In no case shall velocities greater than fifteen (15) fps be allowed.
- E. The sanitary sewer system shall be designed to achieve uniform flow velocities through consistent slopes between manholes.
- F. Minimum slopes shall comply with Table 6-3 and shall ensure a minimum self-cleaning velocity of two and one half (2.5') fps when flowing full.

FIGURE 7: MINIMUM DESIGN SLOPES FOR CIRCULAR PIPES

PIPE SIZE (INCHES)	MINIMUM SLOPE ⁽¹⁾ (FT/FT)
8	0.0050 ⁽⁴⁾
10	0.0025
12	0.0020
14	0.0016
15	0.0015
16	0.0014
18	0.0012
20	0.0010
21	0.0010
24	0.0008

Notes:

- (1) Mains larger than twenty-four (24") inches shall still have a slope no less than eight ten thousandths (0.0008)
- (2) Pipe capacity presented based on full capacity flow
- (3) Table assumes Manning's N value of thirteen thousandths (0.013)
- (4) Minimum slope for an eight (8") inch pipe shall be 0.0050 ft/ft unless specifically evaluated and approved by the Town Engineer

- G. Analyze junction losses and include in design report.
- H. The depth to diameter (d/D) ratio for gravity sanitary sewer mains ten (10") inches in diameter and less shall be no greater than five tenths (0.50) in the ultimate peak flow condition. The d/D ratio for sewer mains greater than ten (10") inches diameter shall be no greater than seventy-five hundredths (0.75) for the ultimate peak flow condition.
- I. Each reach of pipe shall be analyzed for design flow (average day), peak flow, and full flow conditions and placed in a tabular form in the design report. The table shall include average, peak, and full flows, the corresponding velocities, flow depth, and flow depth/diameter ratios for each flow regime.
- J. The Town may require that the wastewater design report include an analysis of the potential of the generation of hydrogen sulfide per EPA guidelines and if so, the design shall provide mitigation.

6.8.6 Manholes & Cleanouts

- A. Manholes in Town of Camp Verde streets shall be located in accordance with TOCV Standard Details 420CV-1 and 420CV-2. Manholes shall not be located in trails,

sidewalks, crosswalks crossings, valley gutter, and curb and gutter. Manholes are required at all changes of grade, pipe size, pipe material or alignment and at distances not to exceed those shown below:

FIGURE 8: SEWER MANHOLE SPACING

PIPE SIZE (INCHES)	MAXIMUM SPACING (FEET)
8 to 10	400
12 to 18	500
Larger than 18	600

- B. All manhole sections and cones shall be pre-cast concrete as detailed in the TOCV Standard Detail 420CV. Manhole bases can be precast or poured in place.
- C. Manhole covers shall be in accordance with MAG Standard Detail 423-1 or 423-2 as required, for applicable size.
- D. Manholes shall be lined with Sewer Shield 150 or equal.
- E. All sewer manholes shall be minimum sixty inches (60”) in diameter with 30 inch (30”) minimum rings and lids.
- F. New manholes over 20’ deep or within major road intersection are to be constructed of polymer concrete.
- G. The upstream pipe shall be the same or smaller diameter than the downstream pipe. Inverts through manholes and junction boxes shall be designed to maintain the energy gradient across the structure. Manholes and junction boxes having sewer mains intersecting at 45 to 90 degrees shall have a minimum of 0.10-foot drop across the structure.
- H. The manhole shall have a minimum one-tenth (0.10) foot drop across the trough. Inverts at junctions shall be designed to maintain the energy gradient across the junction and prevent backflow. Where pipe size increases, through a manhole, pipe crowns shall be matched on each side.
- I. The difference in invert elevations between inflow and outflow mains shall not exceed two (2’) feet, unless a drop connection is installed. If less than two (2’) feet, MAG Standard Detail 420-3 shall be used. Drop connections shall be in accordance with TOCV Standard Detail 426CV.
- J. Existing manholes shall be replaced to accept new mainline connections unless a drop inlet is designed.
- K. Manholes shall be protected from storm drainage and flooding conditions. Sanitary sewer mains shall not be permitted in washes or drainage areas unless otherwise approved in writing, by the Town Engineer.
 - 1. When approved by the Town Engineer:

- a. Manholes located within washes or drainage areas shall have bolted watertight covers, bases and barrels to prevent inflow.
 - b. The rim elevation shall be a minimum of twelve (12”) inches and a maximum of eighteen (18”) inches above adjacent finish grade. Refer to TOCV Standard Detail 420CV-2
 - c. Provide structural protection against scour from a one hundred (100) year storm event
 - d. The manhole shall be designed to structurally, exceed the external forces acting upon the manhole from a one hundred (100) year storm event
 - e. Manholes shall be verified as non-buoyant, or provisions provided to negate the effects of buoyancy
- 2. Ingress/egress and maintenance roads shall be provided for all public sewer mains and appurtenances.
- L. Cleanouts per MAG Standard Detail 441 may be used in place of manholes at the end of mains that 1) cannot be extended, 2) are less than one hundred fifty (150’) feet in length, and 3) have less than four (4) service connections.
 - 1. Service connections are not allowed at the ends of main line cleanouts. Service connections shall be provided off the sanitary sewer main a minimum of two (2’) feet downstream of the cleanout.

6.8.7 Pretreatment Monitoring Vaults & Manholes

- A. The Town Engineer has the sole discretion of when the installation of a monitoring vault and/or sample collection point will be required for non-residential discharges. The following conditions will automatically trigger monitoring point and/or sample collection point requirements:
 - 1. The projected wastewater discharge is equal to or greater than twenty-five thousand gallons per day (25,000 gpd)
 - 2. The projected operation falls under Federal Categorical Classification under 40 CFR Parts 400 – 471
 - 3. The projected operation is otherwise classified by the Town Engineer, the Arizona Department of Environmental Quality (AZDEQ), or the US Environmental Protection Agency (EPA) as a Significant Industrial User (SIU)
 - 4. The projected operation is classified as a Food Service Facility (FSF) or Vehicle Service Facility (VSF) by the Town Engineer
 - 5. Any other discharge regulated under the Town’s pretreatment program
- B. Any developments that meet the criteria in 6.9.7.A. are required to install a monitoring vault and sample collection in accordance with the following provisions:
 - 1. Monitoring vaults shall be constructed with a straight channel and no taps or bends for ten (10’) feet upstream or downstream or as approved by the Town Engineer.

2. Monitoring vaults shall be located and designed for access at all times by monitoring crews and vehicles
 3. Monitoring vaults shall be installed as to prevent inflow from surface runoff
- C. New developments that are classified as food service facilities (FSF) or vehicle service facilities (VSF) by the Town Engineer are required to install a sample collection point that meets the following conditions:
1. The sampling point shall be of sufficient size to allow full access to an opening equal to the inside diameter of the drainpipe
 2. The sampling point shall be accessible to Town of Camp Verde staff during regular working hours
 3. The sampling point shall be installed as to prevent inflow from surface runoff
 4. The sampling point shall be installed immediately downstream of the sand/oil (SO) separator or grease interceptor (GI) and prior to any downstream connection

6.8.8 Pipe Cover & Separation

- A. Sanitary sewer main should be installed, when possible, at a sufficient depth to promote gravity drainage of wastewater from each service line and shall anticipate the lowest potential finish floor elevation for each building pad to the maximum extent possible. The depth shall include the additional height required for service tap placement at the ten (10) o'clock or two (2) o'clock position and an anticipated minimum slope of one quarter ($\frac{1}{4}$ ") inch per foot. The service lateral shall pass below water mains and other utilities where possible and shall not steeply rise within the ROW.
1. In no case shall a sanitary sewer main be installed with less than four (4') feet of cover over the top of the pipe, unless otherwise approved by the Town Engineer.
 2. All sanitary sewer mains and laterals shall be designed to resist superimposed live loads and backfill overburden without damage or deflection to the pipe material and without adversely affecting the hydraulic characteristics of the pipe. The design engineer shall specify minimum depths of cover to be provided during the construction of roadways or other facilities affecting cover over the main and laterals.
- B. Caution shall be taken in the design and construction of the sanitary sewer mains to protect all water supplies from wastewater contamination. To minimize the potential of contamination, the Engineer of Record shall design the horizontal and vertical separation of water, reclaimed water and sanitary sewer mains in accordance with Engineering Bulletin No. 10 "Guidelines for the Construction of Water Systems" published by the Arizona Department of Environmental Quality and the Arizona Administrative Code, Title 18, Chapter 5, "Environmental Reviews and Certification".

1. The minimum horizontal distance from a water or reclaimed water main to a sanitary sewer main shall be six (6') feet, outside to outside. The minimum vertical clearance shall meet MAG Standard Detail 404-1 and 404-2.
 2. Both the water and sewer main shall be constructed of Class 350 (CL350) Ductile Iron Pipe (DIP) with restrained joints.
- C. Sanitary sewer mains shall have a minimum of ten (10') feet of horizontal clearance from any structural footing or substantial improvement. Design shall consider any structural load imposed on the pipe.
- D. Sanitary sewer mains crossing under storm drains or culverts shall be a minimum of two (2') foot below and be adequately protected. Sanitary sewer mains crossing below large structures such as box culverts and bridges, shall require casing pipe, be fully restrained, and have scour protection. Sanitary sewer mains crossing over storm drains shall be a minimum of two (2') foot above and be adequately protected.

6.8.9 Wash Crossings

- A. All wash crossings shall be constructed using restrained joint epoxy coated, Class 350 (CL350) Ductile Iron Pipe (DIP). Bury requirements to place sanitary sewer mains under washes or channels shall be based upon the one hundred (100) year peak design discharge (Q100) in the channel or wash. The minimum depth of bury below the design flow line of the channel or wash shall be shown in Table 6-5, unless site-specific geotechnical and hydraulic analyses are performed to document any reduction.

FIGURE 9: SEWER BURY DEPTH UNDER WASHES

100 YEAR FLOW RATE (cfs)	MINIMUM DEPTH OF BURY (FEET)
1 to 49	5
50 to 99	6
100 to 499	7
Greater than 499	Scour depth based on scour analysis required

- B. Wash crossings with a one hundred (100) year flow above five hundred (500) cubic feet per second (cfs), shall have the scour depth estimated using Arizona State Standard Attachment SSA 5-96, Guideline 2, Level I, as published by the Arizona Department of Water Resources. The design engineer shall estimate the depth of scour and design the top of pipe to be three) feet below the estimated scour depth. The engineer shall provide a detailed analysis of the scour depth with final plans for review and approval.
- C. All pipelines that must be located within the scour zone or do not meet the minimum required depth of bury, as indicated above, shall be protected by installing

a cut-off wall, and energy dissipation downstream of the pipeline to stabilize the scour depth to a minimum of three (3') feet above the pipeline. The design engineer shall design the cut-off wall, energy dissipation, and include details on the improvement plans. The Town Engineer shall review pipe protection and scour stabilization requirements on a case-by-case basis.

6.8.10 Gravity Sanitary Sewer Service Connections

- A. The design engineer shall make every effort to use existing sanitary sewer service connections that have been extended to a property through a “stub out” by previous construction. Where the use of a stub out is not feasible, the existing line shall be abandoned at the sanitary sewer main by installing a glued cap at the wye. Service connections will not be allowed directly into a manhole.
- B. Minimum diameter for a residential service connection is four (4”) inch inside diameter (ID) and six (6”) inches inside diameter for commercial service connections.
- C. All service line connections shall be installed perpendicular to the sanitary sewer main in accordance with MAG Standard Detail 440-1 and 440-2. In addition, service line connections shall be extended to the back of the ROW or public utility easement. No horizontal bends in the service line will be allowed.
- D. Location
 - 1. All proposed service line connections shall be shown on the final plans with stations and dimensions or offsets from the street centerline.
 - a. Each lot or building shall be provided with its own individual service line unless exempt under the Camp Verde Municipal Code.
 - b. Future lot splitting will require a separate service line connection for each lot created.
 - 2. The service line location shall be coordinated to avoid conflicts with other utilities and driveway locations. Locations shall be on the lower elevation side of the lot, whenever possible.
 - 3. A two (2') foot minimum separation between service taps is required.
 - 4. Sanitary sewer lines shall be designed to allow the sanitary sewer service lines to pass under water mains with twenty-four (24”) inches of vertical clearance to minimize potential health hazards.
 - 5. When minimum separation is not possible or the sanitary sewer service line passes over the water or reclaimed water main, the sanitary sewer service line shall be Ductile Iron Pipe (DIP) for a minimum of ten (10') feet on each side of the water main. No joints are allowed within ten (10') feet of the water main.
 - 6. All sewer service laterals shall have a sewer backwater valve installed per the most recently adopted International Plumbing Code (IPC).

- 7. Water and sewer service laterals shall not be installed in the same trench. A minimum six (6') foot horizontal separation shall be maintained.
- E. Sanitary sewer service connections shall be located a minimum of four (4') feet away from a manhole or two (2') feet from a main line cleanout.

6.8.11 Pressurized Sanitary Sewer Service Connections

- A. Pressurized sanitary sewer ejector systems shall be owned, operated and maintained, by the property owner. The Engineer of Record is responsible for the design and hydraulic analysis of the pressurized system.
- B. The Town of Camp Verde does not allow extending private ejector lines across the frontages of adjacent lots or properties except where it is not feasible to extend the Town main due to controlling grades. Any such ejector line shall be placed within a private easement shown on the plat or lot split survey. The developer shall extend the Town of Camp Verde main to provide frontage to the maximum extent possible.

6.8.12 Grease, Oil & Sand Interceptors

- A. The installation of interceptors shall be in accordance with these standards.
- B. Grease interceptors (GIs) shall be installed for all food service facilities (FSFs). Oil and sand interceptors shall be provided for vehicle service facilities (VSFs), laundry facilities, car washes, and other similar facilities. The Engineer of Record shall contact the Town Engineer to determine if an interceptor is required, and what type and size of interceptor is best suited for the proper handling of the projected waste streams. Interceptors shall be installed to be accessible to Town of Camp Verde Utilities Operation staff and vehicles during operating hours.
- C. Each business, restaurant or establishment shall discharge to a separate interceptor. Each interceptor shall be, shown to scale and stationed on the plans.
- D. The Engineer of Record shall coordinate with the mechanical engineer to ensure the following are considered in the design of grease interceptors:
 - 1. Design is compliant with the International Plumbing Code as adopted, Camp Verde Municipal Code (CMC), and approved by the Town Engineer
 - 2. Tank size is appropriate to the maximum projected flow from the establishment and anticipates a ninety (90) day maintenance schedule
 - 3. Potential to develop odors
 - 4. Separation from pedestrian areas or corridors
 - 5. Avoid locating grease interceptor in parking garages, streets and under public parking spaces
 - 6. Ease of cleanup after maintenance and pumping
 - 7. Kitchen garbage grinders shall not be installed
 - 8. Dishwashing equipment shall not be installed as to cause temperatures in the GI to inhibit separation of fats, oil, and grease (FOG) in the interceptor
 - 9. Grease traps will only be allowed in lieu of a GI in very small FSF's

10. Interceptors shall be traffic rated and constructed of steel reinforced precast concrete or other approved rigid corrosion resistant material

6.8.13 Tract & Easement Requirements

- A. All public sanitary sewer mains and appurtenances located outside the public rights of way or a private street tract with easement overlay shall be placed in the center of a minimum twenty (20') foot easement. The sanitary sewer main shall be located a minimum of ten (10') feet from the edge of the easement. The easement shall have legal access from a public ROW, shall be free of obstructions, shall not be located within a fenced area and shall be accessible at all times to Town of Camp Verde service equipment such as trucks, backhoes, etc.
 1. Sewer main depth of ten (10') feet or less: Minimum width shall be twenty (20') feet.
 2. Sewer main depth greater than ten (10') feet: Minimum width shall be two (2) times the depth (centered in easement).
- B. Vegetation within the easement may consist of hydroseed, grasses or other materials, required to comply with erosion control requirements. Trees shall be located outside of the easement.

6.8.14 Alternative Sanitary Sewer Systems

Developers or property owners may request that the Town Engineer consider the use of alternative wastewater systems upon the Engineer of Records determination that conventional gravity or forced sanitary sewer systems cannot provide service to the development without conflicting other provisions of these standards.

7 TRANSPORTATION

7.1 INTRODUCTION

7.1.1 Purpose

The purpose of this Section is to provide transportation and traffic engineering standards, guidelines, and geometric requirements for the design and construction of both public and private project improvements within the Town of Camp Verde.

7.1.2 Applicability

Transportation and Traffic facilities may include arterial, collector, and local streets, alleys, traffic signals, roundabouts, medians, street lighting, street signing, pavement markings, public transit, pedestrian facilities, bicycle facilities, traffic calming devices, pedestrian trails, and other traffic related facilities. The minimum requirements described herein are primarily based on safety considerations; therefore, under most circumstances, standards that provide a greater degree of safety may be used.

7.1.3 Design Standards & Guidelines

The most recently adopted editions of the following design standards and guidelines are approved references and shall be used in conjunction with this Manual.

- A. (AASHTO) The American Association of State Highway and Transportation Officials Policies on Highway Design, Development of Bicycle Facilities, Roadside Design Guide, and Guide for the Planning, Design and Operation of Pedestrian Facilities
- B. (MUTCD) The Manual on Uniform Traffic Control Devices, prepared by the U.S. Department of Transportation
- C. (TOCV) Town of Camp Verde Standard Details
- D. (MAG) The Maricopa Association of Governments Uniform Standard, Specifications and Details for Public Works Construction
- E. (ADOT) Arizona Department of Transportation Manual of Approved Signs
- F. (ITE) Institute of Transportation Engineers Trip Generation Manual
- G. (TRB) Transportation Research Board Highway Capacity Manual

7.2 PLANNING

7.2.1 Traffic Impact Analysis

Access to Highway 260 is regulated by ADOT. Developments directly accessing Highway 260 are required to obtain an encroachment permit through ADOT. Refer to ADOT document 240 Traffic Impact Analysis & Statement.

Access to Town roads will require a Traffic Impact Analysis (TIA) for proposed developments or additions to existing developments that generate more than one hundred (100) vehicle

trips during the morning or afternoon peak hour period. In addition, the Town Engineer may require a TIA when changes in land use, traffic, and roadways occur, such as proposed property rezoning, when the original TIA is more than two (2) years old or where the projected traffic volumes increase by more than ten (10%) percent. The Town Engineer shall make the final determination regarding TIA requirements and the applicable category. The TIA shall be in a format as described in Section 240 Traffic Impact Analysis of the ADOT Traffic Engineering Guidelines and Processes. The TIA shall be prepared under the direction of a Professional Engineer (Civil) licensed to practice in the State of Arizona qualified to prepare TIA reports. There shall be no deviation from these requirements.

A Traffic Impact Analysis Pre-Submittal Form is required for all Subdivision and commercial property development. See Form 240 CV in this section.

7.2.2 Roadway Classification

The Town Engineer shall specify the classification for each street involved in the subdivision plat. Refer to TOCV Standard Details for street typical sections. All land development shall provide for streets in accordance with Town Code, the Planning and Zoning Ordinance and any other planning documents at their normal alignments, widths and geometrics, as determined by the Town Engineer.

State Highway 260 north of I-17 is classified as a Rural Principal Arterial and a Rural Minor Arterial south of I-17. All proposed development with direct access to State Highway 260 will be required to obtain an encroachment permit from ADOT. Development which access State Highway 260 through a collector road will be required to submit a Traffic Impact Statement (TIS) per Traffic Guidelines and Processes (TGPs):

<https://azdot.gov/business/engineering-and-construction/traffic/guidelines-and-processes>

Rural Major Collector roads collect, distribute, and provide direct access to residential, commercial, and industrial properties. These roads have moderate speeds (30mph – 45mph) and may experience a proportionally high percentage of truck traffic.

Rural Minor Collector roads collect and distribute moderate amounts of traffic between arterials and local streets at relatively low (25 – 30 MPH) operating speeds with greater accessibility than major collectors.

The ADOT Functional Classification of Roads can be found at:

<https://www.arcgis.com/apps/mapviewer/index.html?layers=c2a321f877984ceabe64dd5762f2087a>

Local Streets provide direct access to abutting properties. Local streets possess relatively low volumes, operating speeds (25 MPH), trip lengths, and minimal through traffic movements. Local streets shall be arranged as to discourage their use by traffic originating outside the immediate area.

Form 240 CV - Traffic Impact Analysis Pre-Submittal Form

Project Name: _____

Developer/Owner: _____

Phone: _____

Email: _____

Project Location

Brief Description of Project (land use, intensity, timeframe/phasing)

Proposed Access (number, location, restrictions)

Preliminary Assumptions (provide as attachment)

- Trip Generation
- Study Horizon-Years
- Trip Distribution – Peak Hour Traffic
- Study Area Intersections

Traffic Study Preparer

Firm Name: _____

Contact: _____

Phone: _____

Email: _____

Approval by: _____

Date: _____

7.2.3 Roadway Right of Way Requirements

- A. The public ROW requirements shall be based upon recommendations of the Town of Camp Verde's General Plan circulation element, Street Classification Map, and the ultimate needs of the development. The dedicated right-of-way shall provide sufficient area for the installation of utilities, cut and fill of slopes, drainage, postal gang boxes, sidewalks, traffic control devices, access management devices (medians), signs, fire hydrants, landscaping, auxiliary turn lanes, transit facilities and other facilities that may be located adjacent to street corridors.
- B. Standard Details for street typical sections are available on the Town of Camp Verde website.

7.2.4 Intersection Sight Visibility Triangle Easements

Sight visibility triangle easements shall be used as a means to limit the height of structures, vegetation, and other improvements on the portion of property and/or ROW immediately adjacent to intersections, alleys and driveways. These minimum sight visibility triangle easements, however, shall not preclude the design engineer from calculating any additional ROW, easements or sight visibility triangle restrictions required per ASHTO Geometric Design of Highways and Streets

7.2.5 Subdivision Street Planning

- A. The planning of subdivision streets shall discourage through traffic.
 - 1. Street Location and Arrangement
 - a. Street layout shall provide for the continuation of arterial and collector streets in adjacent areas, and shall conform to a standard grid system, when possible, as specified in the EDSM and the Planning and Zoning Ordinance. In certain cases, paved street sections are required beyond the subdivision boundary to provide connectivity to the nearest Town of Camp Verde maintained and paved street.
 - b. Other classifications shall be required to follow a circulating system as well.
 - c. Certain proposed streets, as designated by the Town of Camp Verde Street Classification Map, shall be extended to the sub-division boundary to provide future connection with adjoining un-platted lands. In general, these extensions shall not be farther apart than the maximum permitted length of a block.
 - d. Local streets alignments shall be so arranged as to discourage their use as through routes by traffic originating outside the immediate area.

2. Partial Street Improvements

- a. When a street is required adjacent to and parallel with two (2) subdivision boundaries, the first subdivision to develop shall complete all required improvements. The first subdivision shall submit civil improvement plans to include, in dashed lines, the half street, which will be constructed in the future, and be responsible for the construction of full improvements, at a minimum from their boundary to centerline with the required AC structural section paved width no less than twenty-four (24') feet. Additionally, they shall be required to construct sidewalk, curb and gutter along their development frontage. *(Dependent on ROW width, circulation requirements and length of street, the first sub divider may be required to construct beyond centerline).
- b. For all projects, a full street cross section is required for interior streets and a complete half-street cross section for perimeter streets if the street centerline is the project's boundary line.
- c. The half-street is to be designed to match existing construction as much as possible unless doing so is likely to create an unsatisfactory condition. If changes are needed to correct conditions on an existing half-street to construct the other half of the street, the solutions must be developed in coordination with the Town Engineer on a case-by-case basis. The plans for the new half-street must contain sufficient information on the profile and cross sections of the existing street to demonstrate that the new construction will match the old construction and result in a full street with proper cross sections.

3. Subdivision Blocks

- a. Block lengths, widths and shapes of blocks shall be determined with due regard to:
 - i. Provision of sites suitable to the type of use contemplated.
 - ii. Zoning requirements, related to lot sizes and dimensions.
 - iii. Need for convenient access, circulation, control and safety of street and pedestrian traffic.
 - iv. Limitations and opportunities of topography.
 - v. Circulation within the subdivision, and access to the community facilities.
 - vi. Emergency vehicle access and circulation.
 - vii. Lengths as short as practicable and the discouragement of excessive vehicular speeds, but not to exceed one thousand two hundred (1,200') feet, measured along the centerline of street and between intersecting street centerlines.

- viii. Cul-de-sac lengths less than one thousand three hundred twenty (1,320') feet in length.
- 4. Traffic calming measures can be implemented as part of new communities or retrofitted into existing neighborhoods using the Council adopted Traffic Calming Policy available at the Town of Camp Verde Public Works Department. If a community design calls for traffic calming elements, as required by Town Code, the Town Engineer shall work with the project engineer on suggestions and guidance on acceptable treatments. Traffic calming measures can vary, depending on the specific application. Example traffic calming measures include medians, traffic circles and roundabouts, speed humps, raised crosswalks/intersections, and street narrowing. The Town Engineer must approve the use of traffic calming measures within the Town of Camp Verde ROW.

7.3 HORIZONTAL ALIGNMENT

7.3.1 Horizontal Alignment Design

The horizontal alignment of a road in the Town of Camp Verde shall meet the guidelines set forth in the AASHTO publication A Policy on Geometric Design of Highways and Streets.

- A. Maximum degree of curvature for any subdivision street shall be twenty-five degrees (25°) unless specifically approved by the Town on a case-by-case basis.
- B. Roadway to be constructed equidistant each side from surveyed right-of-way centerline or construction centerline.
- C. All right-of-way centerline deflections over six degrees (6°) shall have a curvilinear alignment.
- D. Deflections less than 6 degrees may have angular right-of-way alignment with the construction centerline to be designed curvilinear. However, no part of the roadway prism may fall outside of the angular right-of-way lines.

7.3.2 Driveway Access Guidelines on Collector Roads or Higher

- A. Driveways shall be designed and located in such a way to provide safe ingress and egress with the fewest number while promoting efficient traffic circulation on the Town of Camp Verde's roadway system.

1. Driveway Spacing

Minimum driveway spacing shall conform to the standards noted in this section unless otherwise approved by the Town Engineer. This minimum spacing applies to proposed site driveway separation, as well as separation from existing or planned driveways on adjacent

parcels and across the street. To provide safe turning movements from driveways on non-residential streets without raised medians, new driveways shall align with existing driveways on the opposite side of the street. Corner parcels may be required to have all access from a side street as determined by the Town Engineer. A non-vehicular access easement (NVAE) may be required to restrict curb cuts to approved locations.

2. Number of Driveways

Table 7-13 and the following shall serve as a guide for the allowable number of driveways on a site unless justified by a Traffic Impact Analysis. Safe driveway requirements per the above section shall be provided at a minimum.

- a. At least one (1) driveway per abutting street shall be allowed unless physical constraints or otherwise directed by the Town Engineer.
- b. One (1) additional driveway may be allowed for a site with continuous frontage of three hundred (300') feet or more on a roadway classified as minor arterial or lower and four hundred (400') feet or more on a major arterial or higher.
- c. Two (2) additional driveways (three (3) total) may be allowed for a site with continuous frontage of more than six hundred (600') feet or more on a roadway classified as minor arterial or lower and eight hundred (800') feet or more on a major arterial or higher.
- d. An additional service type driveway may be allowed for a site under the following conditions:
 - i. Continuous frontage of six hundred (600') feet or more on a roadway classified as minor arterial or lower
 - ii. Continuous frontage of eight hundred (800') feet or more on a major arterial or higher
 - iii. The site layout is such that the service driveway is unlikely to be used by customers of the businesses on the site, and designed as such
- e. Shared access and internal connectivity between adjacent parcels shall be encouraged. Adjoining parcels under common ownership may be required to share a curb cut.
- f. Driveway location must be evaluated with respect to the particular site layout and location. Variations may be permitted where a Traffic Impact Analysis approved by the Town Engineer justifies a departure from these guidelines or an exception is granted through the EDSM, Section 10, process.

3. Driveway Location Limitations

A new driveway shall not be allowed within twenty (20') feet of any commercial property line except when it is a joint use driveway, serving two abutting commercial properties. Commercial driveways shall not be allowed within seventy-five (75') feet of the ROW line of

an intersecting collector street, two hundred (200') feet of an intersecting Minor Arterial, and three hundred (300') feet of an intersecting Major Arterial. Exceptions may be granted through the EDSM, Section 10, process.

Edge of residential driveway on corner lot shall be fifty (50') feet minimum from near edge of pavement.

7.3.3 Median Design

- A. In the interest of public safety, traffic management and street aesthetics, medians are used to separate traffic flow, channelize left turns, control access and reduce conflicts.

- 1. Raised Median

Raised median islands shall be installed in accordance with MAG standards.

- a. Storage Lane Lengths and Tapers: Refer to EDSM, Section 7.4.6 "Auxiliary Traffic Lanes" for minimum specific dimensions, unless the Traffic Impact Analysis results or Town Engineer demonstrates longer lengths are required.
- b. Median curb type: Refer to EDSM, Section 7.6.3 "Median Curb" for requirements.
- c. Termination: Medians shall terminate in a bull nose per MAG Standard Detail 223. Other types may be considered based on the application through the EDSM, Section 10, process.
- d. Full access median openings consist of right in/right out and left in/left out turning movements. Full access median openings may be required to be signalized.
- e. Partial access median openings consist of right in/right out and left ingress only while prohibiting left egress. Partial openings allow fewer traffic conflicts and create a lower potential for collisions.
- f. Spacing for median openings is measured from the center of the median opening to the center of the adjacent median opening or intersection.
- g. All medians shall be signed per the MUTCD guidelines. The beginning of each median where none exists prior, is to be signed. Median breaks in a continuous median are required to have object markers.

- 2. Median Openings

Raised medians on arterial streets are provided to reduce conflicts, channelize movements for safety, and improve traffic flow. It is not possible to provide an opening in the median for every driveway. Careful consideration shall be given to each request for a median opening to ensure that the safety and the intent of the median is not compromised by a proliferation of median cuts. The preference for access along arterial streets is to have full access median openings that align at not

less than one quarter ($\frac{1}{4}$) mile intervals. The design engineer shall line up full access openings in compliance with the street system planned for the arterial corridor. Full access openings shall occur at the mile, one half ($\frac{1}{2}$) mile, and one quarter ($\frac{1}{4}$) mile interval. The preference for access at less than one quarter ($\frac{1}{4}$) mile spacing is to have a partial access median opening; however, these openings are subject to such parameters as safety of operation, flow of traffic requirements for storage, and feasibility of geometrics. These parameters shall be addressed in a Traffic Impact Analysis when requesting a partial access opening. All median openings shall be prohibited within six hundred sixty (660') feet of an arterial-to-arterial intersection. Full access median openings at less than one quarter ($\frac{1}{4}$) mile intervals or within six hundred sixty (660') feet of an arterial-to-arterial, intersection must go through the EDSM, Section 10, process. The applicant shall be required to submit a standard Traffic Impact Analysis that also demonstrates the following:

- a. The full access opening is spaced safely and shall not be in conflict with the planned grid system for the corridor.
- b. The full access opening shall allow for safe operation.
- c. Full-access opening shall not compromise storage requirements.
- d. Full access shall not compromise safety for all other turning movements.
- e. The full access opening shall not significantly affect the flow of traffic on the arterial.
- f. Full access opening will not negatively affect accesses to future property development.

7.4 VERTICAL ALIGNMENT

7.4.1 Vertical Alignment Design

The vertical alignment of a road in the Town of Camp Verde shall meet the guidelines set forth in the AASHTO publication A Policy on Geometric Design of Highways and Streets. Vertical curves shall be designed to provide adequate sight distance, public safety and effective street drainage.

- A. Continuous grades shall not exceed ten percent (10%) unless specifically approved by the Town on a case-by-case basis.
- B. Maximum roadway crown shall not exceed 0.020 feet per foot in cross-section.
- C. Maximum superelevation, or one-way crown, shall not exceed 0.080 feet per foot in cross-section.

7.5 EASEMENTS & DEDICATIONS

7.5.1 Public Utility Easement (PUE)

A minimum eight (8') foot wide PUE shall be located adjacent to each side of the dedicated street ROW for streets classified as Local Streets. Public Utility Easements may not be located within the side or back property lines without the approval of the Town Engineer. Landscaping installed in a PUE shall be of the shallow root, and non-intrusive variety, and shall be maintained by the property owner.

7.5.2 Non-Vehicular Access Easement (NVAE)

Non-Vehicular Access Easement (NVAE) limits vehicular access to a site from arterial, and collector streets. On local streets, a NVAE on private lots is required adjacent to all greenbelts and open space areas. Vehicular access shall be restricted by use of a one (1') foot NVAE, at locations other than street intersections and approved driveways or as requested by the Town Engineer. The ownership and maintenance of the NVAE remains with the property owner of the parcel from which the NVAE is granted. NVAE shall be shown on the final plat.

7.6 CURB & GUTTER

7.6.1 Vertical Curb

- A. Vertical curbs are required on all streets classified as Minor Arterial and higher, Collectors and Local Streets at curb returns, adjacent to common areas, and other areas to restrict vehicle access.
- B. The height of all vertical curb shall be six (6") inch unless otherwise approved by the Town Engineer. Installation shall be per MAG Standard Detail 220-1. Vertical curbs shall be six (6") inch at all medians, and curbs adjacent to landscaping tracts or vertical structures (e.g. cluster mailboxes, utility cabinets, signal equipment, monument signing, etc.).

7.6.2 Median Curb

In locations where raised medians are constructed, vertical curb is required. Median curb shall be installed per MAG Standard Detail 222, Type "A". With Town Engineer approval roll curb may be used around medians installed in low speed, low volume streets, to facilitate truck turning movements with traffic calming projects, or where needed to maintain adequate width for emergency vehicles.

7.6.3 Curb Returns

- A. Vertical curb shall be used through the curb return from point of curvature (PC) to point of tangent (PT) regardless of whether the tangent curb sections are vertical or roll curb.

B. Curb Return Radii on Streets

The radii for curb returns shall be in accordance with Table 7-14: All dimensions are to back of curb.

FIGURE 10: CURB RETURN RADIUS

STREET CLASIFICATION	INTERSECTING STREET CLASSIFICATION			
	MAJOR ARTERIAL	MINOR ARTERIAL	COLLECTOR	LOCAL STREETS
Major Arterial	TBD*	TBD*	35'	35'
Minor Arterial	TBD*	TBD*	35'	35'
Collector	35'	35'	30'	30'
Local Streets	35'	35'	30'	25'

*Curb radii for Arterial Streets shall be determined by speed and a TIA. In all cases, they shall be a minimum of thirty-five (35') feet.

C. Sidewalk Ramp

Sidewalk ramps shall be constructed at all curbed street intersections, at medians with pedestrian refuges, and wherever a pedestrian access route crosses a street. Sidewalk ramps shall align with the sidewalk ramps on the opposite side of the street. If a traffic signal exists or is planned, the sidewalk ramp and apron shall provide access to the pedestrian push button, per ADA requirements.

1. Directional or double sidewalk ramps per MAG Standard Details 236-1 through 236-5 shall be installed at all arterial and collector street intersections. Where directional sidewalk ramps are required, the minimum curb return radius shall be thirty (30') feet.
2. Single sidewalk ramps per MAG Standard Details 236-1 through 326-5 shall be installed at all local street intersections.
3. At T-intersections one sidewalk ramp shall be installed per MAG Standard Details 238-1 through 238-3. The ramp shall normally be placed on the right-hand side of the TEE and align with ramps on the opposite side of the street.
4. Along safe routes to school, any crosswalk location or trail connections, additional sidewalk ramps may be required.
5. Projects that include construction improvements at existing street intersections where sidewalk ramps are located shall note whether the ramps comply with current Town of Camp Verde standards. If the sidewalk ramps are not in compliance, they shall be removed and replaced with sidewalk ramps that meet Town of Camp Verde standards.

7.7 SIDEWALKS

Installation of sidewalks shall promote and enhance pedestrian safety and the aesthetic quality of the roadway. Streets constructed to Town of Camp Verde standards shall have sidewalks installed per MAG Standard Detail 230 and Town of Camp Verde Standards. Sidewalks shall remain within the ROW.

7.8 PAVEMENT CROSS SECTIONS

7.8.1 Undivided Streets

Undivided streets should have a normal crown that is a two-way cross slope with the cross section high point on the street centerline. A raised crown with a constant cross slope of two (2%) percent is required on all public streets. Inverted crown sections are not allowed, except as required at arterial or collector street intersections and alleyways.

7.8.2 Divided Streets

Divided streets should have cross slope on each pavement section. The high point of each slope on each pavement section must occur on the edge of the pavement nearest to the median. Unusual conditions may cause cross slope requirements to vary, but normally, the desirable cross-slope is two (2%) percent. The desirable slope from top of median curb to top of median curb shall not exceed fifteen (15%) percent.

7.9 CUL-DE-SACS & KNUCKLES

7.9.1 Cul-De-Sac Street

- A. A cul-de-sac street is a street that serves more than one (1) property owner and has only one (1) direct access to the public street system.
- B. Cul-de-sac streets shall be a maximum of one thousand three hundred twenty (1,320') feet in length and terminate in a circular turnaround.

7.9.2 Knuckles

Knuckles are areas on the roadway expanded to provide a turn-around and additional access or lot frontage on local streets. Knuckles are permitted to improve accessibility to odd-shaped sites. The use of knuckles (except on a cul-de-sac) on other than local streets must be approved by the Town Engineer.

7.10 DEAD END STREETS

Dead end streets shall be prohibited except short stubs where a street connection is necessary to serve adjacent un-platted land that will develop at a future date. Temporary turnarounds shall be required where the street stub exceeds one (1) lot or one hundred (100') feet in length. The developer shall provide a sign at the stub declaring that the particular street shall connect with any future development.

7.11 MAILBOXES

A. Curb Side Mailboxes:

1. Curb side mailbox with Parkway: When an open space exists between the back of curb and the sidewalk of three (3') feet or more (parkway), the mailbox shall be installed so that the front is flush with the back of curb. The mailbox shall be a minimum of forty-two (42") inches above ground, and on a two (2") inch diameter steel or four (4"x4") inch wood breakaway post.
2. Curb side mailbox behind sidewalk shall not be permitted.
3. Masonry mailboxes will not be permitted within the street ROW.

B. Cluster Mailboxes:

Cluster style mailboxes: a style whereby mailboxes, meeting the specifications of the United States Postal Service, are assembled and grouped together in a single area of land so that they are regarded as one unit. Requirements:

1. The location of a cluster box shall be approved by the Town Engineer and the local, United States Postal Service. For cluster boxes in new subdivisions, the location of the unit shall be shown on the preliminary plat.
2. No driveway access shall be constructed within ten (10') feet of the cluster mailbox.
3. Units shall not be installed within one hundred (100') feet of a street intersection. Units shall not be installed in locations where, because of the position of, shape or color it may interfere with, obstruct the view of or be confused with any traffic sign, signal or device.
4. A vehicular pullout area shall be provided at all cluster mailbox locations to not impede normal traffic of the primary roadway used for access.
5. Cluster mailboxes shall be located on property lines or on a dedicated open tract on the same side of the street designated as a "no parking" zone and in a manner as to provide sufficient sight distance for pedestrian access.
6. A sidewalk shall be constructed perpendicular to the curb adjacent to the unit providing a pedestrian connection from the unit to the curb.
7. The cluster box unit shall be located on a concrete pad and the unit shall be setback a minimum of one (1') foot from the back edge of the sidewalk to the nearest edge of the unit.
8. Cluster mailboxes units shall not be located to conflict with any public utilities, including but not limited to manholes, fire hydrants, meters, vaults and cabinets.
9. When in the opinion of the Town Engineer it is necessary for pedestrian and vehicular safety, an access driveway turnout shall be required at the developer's expense, built three (3') feet outside of the face of curb.

7.12 TRAFFIC CONTROL DEVICE SELECTION

- A. Traffic control device selection is critical to the successful operation of the transportation network. Several options exist for controlling traffic at intersections, but this section shall focus on the two (2) highest levels of control, which include traffic signals and roundabouts. These are often interchangeable and the use of one over the other is not always evident. The following information provides guidance for the design Engineer on choosing one over the other. This justification procedure shall be documented in the Traffic Impact Analysis for each intersection requiring all way stop control or traffic signalization.

1. Installation Warrants:

The studies required to justify the installation of a traffic signal are based on the warrants and requirements set forth in the Manual of Uniform Traffic Control Devices (MUTCD). No such warrants or requirements exist for roundabouts. However, roundabouts shall be considered as an alternative to traffic signals and all way stop control whenever they are warranted.

2. Roundabout Criteria:

Roundabouts shall be considered the most appropriate form of control at any intersection if three general questions can be answered positively.

- a. Will a roundabout be expected to perform better than other alternative control modes? In other words, will it reduce delay, increase capacity, reduce queue lengths or solve some other operational problem?
- b. Will a roundabout provide improved safety over other alternative control devices?
- c. Will a roundabout be expected to provide improved operational characteristics over other alternative control devices? This includes adjacent access operations and emergency vehicle operations.

If it is determined a roundabout is the most appropriate form of control based on the operational criteria; the final stage is to compare the construction cost, life-cycle cost, ROW considerations, “reserve capacity” (the ability to accommodate traffic growth), and constructability. If no clear disadvantage is evident with the roundabout, then it shall be moved forward as the preferred traffic control device.

7.13 ON-STREET PARKING

A. Sign Spacing:

Signs restricting on-street parking shall normally be placed in accordance with the distances shown in Table 7-4. These may be adjusted as required by the Town Engineer when:

1. An assessment of the anticipated parking demand indicates a change is needed
2. The roadway vertical or horizontal alignment reduces the visibility of the signs
3. Parking is allowed in some areas and not in others to indicate the beginning and ending of no parking zones. This can be accomplished using “Begin” and “End” placards, arrows or specific wording on the signs. This also applies when parking time limits are used, or handicap spaces are designated, in which case individual stalls shall be signed.
4. For special conditions such as hydrants, sight visibility requirements, special restrictions needed for traffic control devices, etc.

FIGURE 11: PARKING RESTRICTION SIGN SPACING

STREET CLASSIFICATION	MINIMUM SIGN SPACING (ft)	MAXIMUM SIGN SPACING (ft)
2-Lane Collector	200	300
Local Streets	150	250

B. Parking Restriction Guidelines:

On-street parking shall be restricted on those roadways with insufficient width to allow both parked vehicles and the safe, efficient movement of through traffic as shown in Table 7-5.

FIGURE 12: PARKING RESTRICTION GUIDELINES

ROADWAY PAVEMENT WIDTH (ft)	NO PARKING ALLOWED	PARKING ON ONE SIDE	PARKING ON TWO SIDES
< 28	X		
≥ 28 to < 36		X	
≥ 36			X

* Parking guidelines assume bike lanes are not used.

C. Sign Type:

Signs restricting on-street parking shall be of the type and size in accordance with those shown in Table 7-6. Alternative types and sizes shall only be approved for special circumstances as approved by the Town Engineer.

FIGURE 13: PARKING RESTRICTION SIGN TYPE & SIZES

STREET CLASSIFICATION	APPROVED SIGN	SIZE	PLACARD OR ARROW	ORIENTATION TO TRAVEL WAY (degrees)
4-5 Lane Arterial	R8-3	30" x 30"	N/A	90
3-4 Lane Collector	R8-3	24" x 24"	N/A	90
3-4 Lane Collector	R7-9	12" x 18"	N/A	90
2-Lane Collector	R7-1	12" x 18"	Begin & End	90
			Here to Corner	30 to 45
Local Street	R7-1	12" x 18"	Begin & End	90
			Here to Corner	30 to 45

7.14 BUS BENCH / SHELTER LOCATIONS

- A. When properly located, adequately designed, and effectively enforced, bus stops can improve service and expedite general traffic flow. Decisions regarding bus stop spacing and location call for a careful analysis of passenger service requirements (demand, convenience, and safety), the type of bus service provided and the interaction of stopped buses with general traffic flow. The following guidelines provide direction for the design Engineer for the installation of bus stops in the Town of Camp Verde.

1. Stop Spacing

Bus stops are locations where passengers access the transit system. Bus stops must therefore be convenient to the places where passengers wish to go. Convenience and speed must be balanced in determining appropriate bus stop placement, as too many bus stops can slow down travel times and impede traffic movements. The following guidelines on bus stop spacing shall apply:

- In the downtown area, bus stops shall have a spacing of eight hundred (800') feet to one thousand three hundred (1,300') feet.
- Outside the downtown area, bus stops shall be spaced no less than one thousand five hundred (1,500') feet apart.

2. Bus Stop Locations

Bus stops shall be placed along the street curb for direct safe passenger access to and from the sidewalk, waiting and walking areas. Bus stops are placed in one (1) of three (3) locations: far side (located immediately after an intersection); nearside (located immediately before an intersection); and mid-block (located between intersections). Each of these locations offers advantages to vehicle drivers and pedestrians. However, in general the following guidelines apply to their use:

- a. Far-side stops are the preferable choice for service because they reduce conflicts between right-turning vehicles and stopped buses, eliminate sight distance deficiencies on approaches to an intersection, and encourage pedestrian crossing at the rear of the bus.
 - b. Nearside stops shall be avoided whenever possible. However, they may be considered on a case-by-case basis if in the opinion of the Town Engineer a far-side stop is deemed unsafe or impractical.
 - c. Midblock stops are considered special case stops and are to be used only in special circumstances when in the opinion of the Town Engineer there is a special need based on trip attractors, transfer points or other special demands that cannot be accommodated at an intersection nearby. Bus turnouts shall be used for midblock stops.
3. Bus Stop Benches
- a. Each bus stop location shall consist of one bench or shelter unit unless otherwise approved. Multiple benches at one stop location may only be considered when ridership records indicate a clear need for their installation.
 - b. Bus benches shall be placed in a manner that sidewalk passage is not narrowed to less than the minimum widths set forth by the Americans with Disabilities Act.
 - c. Bus bench placement, which would result in sidewalk widths less than thirty-six (36”) inches, shall require the transit provider to install additional sidewalk width and acquire additional ROW at their expense.

7.15 TRAFFIC SIGNALS & LIGHTING

7.15.1 Public Street Lighting

Developers of all residential, commercial, industrial or other types of properties are responsible for the design and installation of street lighting on all streets within and adjacent to their sites when required by the Camp Verde Municipal Code. Street light plans shall be prepared and sealed by a licensed electrical engineer registered in the State of Arizona. The street lighting design shall be reviewed and approved by the Town and shall take into account “dark sky initiatives” in recommendations on the number, spacing, and

shielding of new street lighting. The street light design shall include numbering of the street light poles as directed by the Town Engineer, ADOT, and Arizona Public Service.

7.15.2 Street Light Selection

These street lighting criteria and guidelines apply for all standard installations.

7.15.3 Street Light Design

The developer shall retain a Professional Electrical Engineer, registered in the State of Arizona, to prepare the lighting system design and appropriate calculations relative to illumination levels. Illumination design shall follow the recommendations of the American National Standard Practice for Roadway Lighting, Illuminating Engineering Society of North America, IES RP-8. The Luminance criteria shall be used to determine compliance with Town street lighting design guidelines.

7.15.4 General Street Light Notes

- A. Streetlights shall normally be located on property lines whenever possible, at least five (5') feet from driveways or any above ground facility, and at such locations to maximize their separation from trees.
- B. Streetlights shall be located on the outside edge of the curve for horizontal curvilinear street alignments.
- C. The minimum curb overhang by street light luminaries (excluding historic globe style) shall be no less than two (2') feet.
- D. Where the sidewalk is separated from the curb (parkway exists), the street light pole shall be centered two (2') feet from the gutter flow line.
- E. Where the sidewalk is contiguous to the curb and the sidewalk the street light pole shall be placed one and one half (1 ½') feet behind the sidewalk.
- F. Street trees can significantly interfere with the efficient lighting of streets and shall not be permitted within thirty (30') feet of any street light standard.
- G. All electrical connections and supply shall be placed underground.

7.15.5 Traffic Signals

This section shall present the criteria and procedures to be utilized by consultants when performing traffic signal work in and for the Town of Camp Verde.

- A. Traffic Signal Spacing:

Street Intersection Spacing on State Highways and Arterials:

- 1. Minimum one half (½) mile spacing shall be required for all warranted traffic signal intersections on all state highways, parkways and major arterial in all cases, except where subject to a highway access management plan or as otherwise approved by ADOT. Consideration for intersection spacing as

close as one quarter (¼) mile may be considered through the EDSM, Section 10, process. Recorded easements for shared access, provision of acceleration/deceleration lanes, traffic signals and other ROW improvements, for these cuts may be required prior to approval.

2. Minimum one quarter (¼) mile spacing shall be required for all warranted traffic signal intersections on Arterials, 3-4 Lane Collectors, and 2-Lane Collectors in all cases. Consideration for intersection spacing as close as one quarter (¼) mile to six hundred sixty (660') feet may be considered to provide required access through the EDSM, Section 10, process. Recorded easements for shared access may be required prior to approval. Adjoining parcels under common ownership may be required to share a curb cut.

B. Traffic Signal Design Guidelines:

The following publication most recent editions and approved supplements by the State and Town of Camp Verde are to be used in conjunction with the design criteria when designing traffic signals for the Town of Camp Verde.

1. Manual on Uniform Traffic Control Devices for Street and Highways; USDOT/FHWA
2. Traffic Signals and Lighting (Standard Drawings); ADOT
3. ADOT Standard Specifications for Road and Bridge Construction
4. International Signal Association Specifications
5. Equipment and Materials Standards of the Institute of Transportation Engineers

7.16 SIGNING

7.16.1 Signing

In new development, the applicant shall install all traffic control signs, which shall include but not be limited to street name, parking, stop, warning, dead end and pedestrian signing. The applicant shall be responsible for supplying and installing the required signs, posts and hardware in accordance with the required standard plans and specifications.

7.16.2 Sign Design Guidelines

The following publications most recent editions and approved supplements by the State and Town are to be used in conjunction with the design criteria when designing traffic signing for the Town of Camp Verde.

- A. Manual on Uniform Traffic Control Devices for Street and Highways (MUTCD); USDOT/FHWA
- B. Manual of Approved Signs (ADOT)
- C. American Association of State Highway Transportation Officials (AASHTO)

7.16.3 General Signing Guidelines

- A. All signing shall conform to the most recent editions of the publications shown above with regard to size, color, shape and placement.
- B. All signs shall be new other than those shown on the plan to be relocated. All new and relocated signs shall be mounted on new posts with new hardware. Signs designed for installation on existing street light poles shall be mounted with new hardware.
- C. Traffic sign dimensions, colors and lettering shall conform to the latest MUTCD specifications. Traffic sign size shall be standard unless otherwise specified here or on the plans.
- D. All signs shall be located at least one (1') foot from the curb face to the nearest edge of the sign. All other roadways signs shall be mounted from six (6') feet to twelve (12') feet from the edge of the pavement to the nearest edge of the sign.
- E. Roadways with guardrail signs shall be located at least six (6') feet from the face of the guardrail to the nearest edge of the sign.
- F. Sign location shall be coordinated with landscaping plans to ensure sign visibility per AASHTO standards.
- G. Signs shall be mounted on street light poles whenever feasible.
- H. All signs installed in areas where parking or pedestrian movements occur shall be erected at a height of seven (7') feet above the pavement or sidewalk to the bottom of the sign or to the lowest sign in a multiple sign installation assembly with the following exceptions:
 - 1. The height to the bottom of a secondary sign mounted below another sign may be up to one (1') foot less than the height specified above in rural areas where pedestrians are not expected.
 - 2. Object markers shall be installed at least four (4') feet above the pavement.
- I. All R1-1 "STOP" signs and pedestrian warning signs shall be retro-reflective with all sheeting material to be diamond VIP grade, meeting or exceeding ASTM 4956-04.
- J. All other signs are to be retro-reflective with all sheeting material to be high intensity prismatic meeting or exceeding ASTM 4956-04.
- K. Sign blanks shall be 5052-H38 alloy treated aluminum with Alodine 1200 conversion coating, 0.080" thick with rounded corners.
- L. Stop signs are to be shown at all local street intersections within a subdivision unless an engineering study shows that no control or yield control is warranted. Stop signs shall be designed and shown on the minor leg of all collector and non-signalized arterial street intersections.
- M. "STOP" signs and "YIELD" signs shall be a minimum of thirty (30") and thirty-six (36") inches in width, respectively. When specified by the requirements of the MUTCD, thirty-six (36") inch and/or forty-eight (48") inch signs may be required on major collectors and arterial streets.

- N. At intersections where all approaches are stop controlled, “All Way” placards shall be placed below the R1-1 stop signs.

7.16.4 Sign Posts

- A. Sign posts shall conform to ADOT Signing Standard Drawing S-3 with the modifications outlined below.
- B. For new construction, galvanized steel, four (4) sided perforated square tubing is required. Refer to ADOT Signing Standard Drawing S-2 for larger sign post selection.
- C. The base and sleeve system for the sign shall be anchored in a minimum of a twenty-four (24”) inch deep, twelve (12”) inch diameter foundation of concrete. The base shall have a breakaway slip base system.

7.16.5 Street Name Signs

- A. All reflective sheeting material shall be high intensity prismatic, meeting or exceeding ASTM 4956-04.
- B. All transparent acrylic, pressure-sensitive film shall be 3M#177 green electro cut film or approved equal.
- C. Font shall be uppercase four (4”) inch minimum.

7.17 STRIPING

7.17.1 Striping

New plats and developments shall install all striping and pavement legends as determined by the Town Engineer, which shall include but not be limited to lane lines, crosswalks, stop bars legends, and arrows. The applicant shall be responsible for supplying and installing the required striping in accordance with standard plans and specifications.

7.17.2 Striping Design Guidelines

- A. The following publications’ most recent editions and approved supplements by the State and Town are to be used in conjunction with approved design criteria when designing striping for the Town of Camp Verde.
 - 1. Manual on Uniform Traffic Control Devices for Street and Highways (MUTCD); USDOT/FHWA
 - 2. American Association of State Highway Transportation Officials (ASSHTO)
- B. General Striping Guidelines
 - 1. Striping shall conform to the most recent editions of the MUTCD specifications with regard to size, color, reflectivity and placement unless otherwise specified here.
 - 2. All roadways classified as Collector or higher shall require centerline striping, stop bars, crosswalks and other appropriate legends and arrows. Local streets are normally exempt.

3. All thermoplastic applications shall conform to ADOT specification 704. Transverse markings, symbols and legends shall be 90 mil (0.090 inch) thick, longitudinal markings shall be 60 mil (0.060 inch) thick alkyd extruded thermoplastic.
4. All paint application shall conform to ADOT specification 708.
5. All conflicting striping, pavement markings, and curb paint shall be removed by wet sandblasting or other approved method prior to the installation of new striping. Slurry or paint shall not be used to cover existing paint. Pavement that is damaged due to the removal of markers or striping shall be repaired to the satisfaction of the Town Engineer.
6. Through lanes shall be a minimum of twelve (12') feet in width, with the exception that lane(s) of eleven (11') feet minimum are used on the center lane of a Major Arterial Street.
7. Turn lanes shall equal that of the through lanes but shall be at least ten (10') feet wide.
8. Parallel parking lanes adjacent to the curb with a gutter pan shall be a minimum of five (5') feet in width from the edge of gutter.
9. A maximum of two (2') foot offset in the lane striping through an intersection is acceptable.
10. All skip lane lines and edge of road markings shall be four (4") inches in width unless otherwise specified.
11. Four (4") inch solid white edge lines shall be used on streets that do not have curb and gutter.
12. All skip white or yellow road markings shall be a ten (10') foot stripe with a thirty (30') foot gap (forty (40') foot repeated interval).
13. Crosswalks shall normally be twelve (12") inches in width.
14. Stop bars shall be eighteen (18") inches wide and set back two (2') feet from the crosswalk, or in the absence of a crosswalk placed at the desired stopping point. They are required at all intersection locations where crosswalks are used and at all stop-controlled intersections on streets classified a Collector and higher. Additionally stop bars are required at all Local Street intersections that intersect streets classified as a Collector and higher.
15. Dual turn lanes shall require the installation of lane delineation markings, which shall consist of white six (6") inch wide thermoplastic skips placed in a two (2') foot stripe with a four (4') foot gap pattern.

C. Crosswalks

Crosswalks shall be required at all legs of signalized intersections and roundabouts and may be required at other locations in yellow or white, standard or high visibility types as determined by the Town Engineer.

1. A white twelve (12”) inch transverse crosswalk shall be used at all designated pedestrian crossings that require the minimum visual treatment.
2. A white crosswalk with twelve (12”) inch diagonal and longitudinal marking shall be used on collector streets with higher volumes to provide increased visibility to drivers. This will result in a more noticeable pavement marking than the standard white crosswalk.
3. A white crosswalk with twenty-four (24”) inch transverse and twelve (12”) inch longitudinal markings provide the highest visibility to drivers and shall be used on arterial streets with the greatest volumes, requiring a more noticeable pavement marking than the diagonal white crosswalk.
4. A yellow crosswalk with twelve (12”) inch transverse and optional diagonal markings shall be used at designated school crossings. Its twelve (12”) inch wide diagonal markings help to provide increased visibility to drivers and is the standard school crossing on streets with lower volumes.
5. A yellow crosswalk with twenty-four (24”) inch transverse and twelve (12”) inch longitudinal markings shall be used at designated school crossings on streets classified as Major Collector and higher, requiring a more noticeable pavement marking than the diagonal yellow crosswalk.

D. Speed Hump Markings

The markings shall be white extruded or precut thermoplastic (90-mil thickness) per MUTCD.

E. Bike Lane Marking

1. Bike lane symbols shall be installed as part of on-street bike lane striping at the beginning of each block and at a spacing of six hundred (600') feet thereafter in the absence of intersecting streets.
2. Bike lane striping lines shall be six (6”) inches in width, solid white unless otherwise specified.
3. Bike lanes shall be a minimum of four (4') feet in width when placed adjacent to the lip of gutter and five (5') feet in width when placed between a travel lane and a parking aisle.

7.18 GUARDRAIL

All new developments shall provide for the design and construction of guardrail as determined by the warrants, procedures and policies in the AASHTO Roadside Design Guide. In areas where short sections of guardrail are warranted, the developer shall design and construct additional length sufficient to ensure that the guardrail system has collective post strength to resist failure or total displacement of the system.

8 DRY UTILITIES

8.1 GENERAL INFORMATION

8.1.1 Purpose

The purpose of this section is to provide guidelines for the location and installation of private utilities within dedicated Town ROW, private streets, and PUE's. This section also provides criteria to integrate private utilities with Town water and sewer.

8.1.2 Applicability

This Section will identify the minimum criteria for design, permitting, construction and installation of private utilities in dedicated Town ROW, private streets and PUE.

8.2 DEDICATION OF PUBLIC UTILITY EASEMENTS

The developer shall dedicate all public utility easements necessary to provide utility service to and through the proposed project. All PUE's shall be recorded on the plat, survey, or by separate instrument in the office of the Yavapai County Recorder.

8.3 BLUE STAKE NOTIFICATION (ARIZONA 811)

A Blue Stake notification shall be placed on each sheet of the plans. Prior to excavation in the Town ROW or PUE, the contractor shall call Blue Stake (1-800-STAKEIT or 811) for the location of all underground utilities.

8.4 UNDERGROUND CONFLICTS

New and existing private utilities within a proposed development shall be placed underground. Transformers, switching cabinets, terminal boxes and 69 KV power transmission lines or larger may be placed above ground, within the PUE. Other locations will require the approval of the Town Engineer.

8.5 CODE CONFLICTS

Town of Camp Verde Municipal Code, Camp Verde Planning and Zoning Ordinance, and Maricopa Association of Governments (MAG) may also apply to the installation and construction of private utilities. If a regulatory conflict exists, the code, which is most restrictive, shall apply.

8.6 CONFLICT REVIEW

- A. Private Utilities located within the Public Utility Easement (PUE) or crossing Town ROW will require a conflict review by the Town Public Works Division prior to the issuance of a ROW Permit.

- B. Construction plans for private utility facilities located within Town ROW, Private Street, or PUE must be submitted to Public Works Division for review and approval to ensure that the integrity and minimum separation of both the public and private utilities is maintained.

8.7 PLAN REQUIREMENTS & GUIDELINES

It is recommended that a meeting be scheduled with the Town to review requirements. Minimum standards of the plans shall meet the following:

- A. Identify and dimension all existing and/or proposed utilities in the Town ROW and PUE.
- B. Show all existing rights-of-way and/or proposed PUE(s).
- C. Show existing and/or proposed street improvements. (e.g. pavement, curb, gutter, sidewalk, driveways, street light poles, water meters, fire hydrants, traffic signals and signal cabinets).
- D. Identify and dimension all existing and/or proposed Town of Camp Verde public utilities (e.g. water, sewer, drainage and streetlights). To avoid conflict, underground utilities shall be shown on the profile with depth elevations and stationing. Profile shall be on the same sheet as the plan view, aligned with the matching station. Utilities may have different stationing, in this case reference the corresponding street station.
- E. Identify and dimension all underground and above ground facilities including those owned by other providers within the Town ROW or PUE.
- F. Minimum separation between non-Town private utilities and Town of Camp Verde public utilities is four (4') feet horizontal, and two (2') feet vertical.
- G. Submit plans on pdf to the Camp Verde Community Development Division including a traffic control plan in accordance with MUTCD along with the right-of-way application for review by the Public Works Division.

8.8 PERMIT REQUIRED

Prior to issuance of a permit to allow the excavation and installation of private underground utilities (e.g. power, telephone, natural gas, cable TV and fiber optic) in any Town ROW, private street or PUE, the developer or contractor installing underground private utilities shall apply for and receive a ROW Permit from the Public Works Department. The permit request shall include construction plans and a Traffic Control Plan. All work performed as part of the ROW Permit shall conform to the requirements of this section.

8.9 COMMON TRENCH & LOCATIONS

- A. On local streets, private utilities (utilities other than water, sewer, and storm sewer), except natural gas and electric, may be located in a common trench behind the curb

and within the Public Utility Easement (PUE). Exceptions to using a common trench and location would require approval by the Town Engineer.

- B. On collector and arterial streets, with sidewalks on both sides of the street, private utilities, except natural gas and electric, may be located in a common trench behind the sidewalk and within the PUE, on the side of the street with the least conflict with Town utilities. Exceptions to using a common trench and location would require approval by the Town Engineer.
- C. Natural gas and electric shall be located in a separate trench within the Town ROW, PUE or private streets on the opposite side of the street of the other private utilities. Exceptions to this location would require approval by the Town Engineer. Natural gas may be placed in a common trench with other utilities, with written approval from all utility franchisees within the Town.
- D. Trench and backfill for all underground private non-Town utilities shall conform to TOCV Standard Detail 200CV.

8.10 MINIMUM CLEARANCES

All proposed private utility facilities shall maintain the following minimum clearances between Town of Camp Verde utilities:

- Horizontal – Four (4') feet
- Vertical – Two (2') feet

8.11 PAVEMENT CUTS

- A. There shall be no pavement cutting on public streets with paved surfaces less than seven (7) years old.
- B. Open trenching (pavement cut) is allowed on streets with paved surfaces that are a minimum of seven (7) years old. Backfill and pavement replacement shall conform to TOCV Standard Details 200CV.
- C. Pavement patching for the street cuts shall be made within twenty-four (24) hours of completion of work as specified on TOCV Standard Detail 200CV.
- D. Depending on the condition of existing pavement, additional removal and replacement of pavement wider than the trench may be required. When the trench of any lineal utility project is within three (3') feet or less from lip of gutter, the pavement area between the trench and lip of curb will also require full removal and replacement. Pavement replacement shall be the minimum structural section for said street classification. In no case shall the replacement of existing asphalt be less than three (3") inches asphaltic concrete (AC) over six (6") inches aggregate base course (ABC) (the minimum residential section). In addition, should any raveling or damage occur to the existing pavement within the construction area, the damaged areas shall be saw-cut, removed, and replaced. Damage to curb, gutter and sidewalk shall require full panel removal with saw cuts made at each joint.

Removal and replacement of any and all existing infrastructure, damaged pavement, concrete, landscaping and irrigation, etc. shall be at the contractor's or private utility's expense.

8.12 HORIZONTAL BORES

In lieu of pavement cuts, developers and contractors may use boring and jacking. Designs with pavement boring shall include a boring profile to ensure proper separation is maintained from all existing utilities. Profiles of existing shall be potholed prior to start of work and included on the boring profile. Utility potholes shall remain open with plate at critical crossings to visually verify depth of bore and avoid utility conflicts.

8.13 DRY UTILITIES CHECKLIST

- A. Dry Utility Plan sheets shall be submitted in accordance with the EDSM, Section 9. Plans shall provide at a minimum the following:
 - 1. Property lines.
 - 2. ROW lines.
 - 3. All recorded and proposed PUE(s).
 - 4. Identify and dimension all existing and proposed private utilities.
 - 5. All existing and proposed infrastructure including curb, gutter, sidewalk, pavement, water, water meters, fire hydrants, wastewater, drainage culverts and catch basins, street light poles, traffic signals, signal cabinets.
 - 6. Traffic Control Plan.

9 SURVEY

9.1 INTRODUCTION

9.1.1 Purpose

The intent of this section is to ensure all projects that involve topographic, boundary, control, as-built, and ALTA surveys; aerial mapping survey ground control; construction staking; ROW and easement investigation; preparation of legal descriptions and exhibits; and improvement plans will apply consistent methods and standards pertaining to ground surface measurement within the Town of Camp Verde.

9.1.2 Applicability

All survey work in the Town of Camp Verde will be performed under the direction of a professional land surveyor registered in accordance with requirements of the State of Arizona Board of Technical Registration at <http://www.btr.state.az.us/>.

9.1.3 Statement of Intent with regard to Digital Data

The Town of Camp Verde requires digital CADD data for all submittals. It is the Town's intent to use this base form data in-house to benefit the public by updating GIS models, Town Survey Models, and As-Built Records with increased accuracy and efficiency. The Town shall not release proprietary CADD format digital information in base form without the consent of the design engineer or registered land surveyor. The Town shall have the right to release IMAGE files of plan set data to the public as public record information in the same way that hard copy plan files are released.

9.1.4 Geographic Information System (GIS) Technology

The Town is using GIS technology for capturing, managing, analyzing, and displaying all forms of geographically referenced data and information. GIS data submitted to the Town shall comply with the Arizona Spatial Data Accuracy and Geo-Referencing Standards available from Arizona Professional Land Surveyors (APLS).

9.2 LAND SURVEYING

9.2.1 Boundary Survey Standards

- A. Boundary Surveys: All land survey work will be performed within the guidelines of the Arizona Boundary Survey Minimum Standards. These standards may be obtained from the State of Arizona Board of Technical Registration at 1110 West Washington Street, Suite 240, Phoenix, Arizona 85007, or online at: <https://btr.az.gov/laws-standards/standards/standards>.

- B. ALTA Surveys: ALTA surveys will be based on American Land Title Association, American Congress on Surveying & Mapping, and National Society of Professional Surveyors, latest edition, Minimum Standard Detail Requirements for ALTA/ACSM Land title Surveys:
- http://c.ymcdn.com/sites/www.nsps.us.com/resource/resmgr/ALTA_Standards/20110223ALTAACSMLandTitleSur.pdf?hhSearchTerms=%22minimum+and+standard+and+detail+and+alta%2facsm%22.

9.2.2 Horizontal & Vertical Control System

- A. Survey Datum Requirements. When submitting survey data for engineering plans that are to be submitted to the Town of Camp Verde, the data must meet the following survey datum requirements:
1. Units of measure shall be in international feet for horizontal and vertical coordinates, North American Vertical Datum of 1988 (NAVD88) for vertical, and State Plane (Central-International feet) coordinates for horizontal. Please refer to Table 9-1.
 2. A survey block or note listing at least two on-site points conforming to “Town of Camp Verde Survey Datum Requirements” must be provided. These two points must have a Northing, Easting and a NAVD88 elevation.
 3. Centerline monuments shall be at all Points of Curvature (PC), Points of Tangency (PT) and intersections for horizontal curves. ROW monuments should be a rebar in concrete at PC’s, PT’s and angle points.
 4. All plats shall be submitted in digital CADD format on the Town of Camp Verde Datum as set forth herein for Town of Camp Verde review at the preliminary submittal stage, as well as for final plat acceptance.
 5. All CADD files shall be on a single disk media in AutoCAD (a release currently supported by Autodesk subscription plans). CADD files shall consist of base/design files on the Town coordinate system to include, but not limited to the following: Topography, ROW, Sewer, Water, Drainage, Grading, Typical sections, etc. All deliverables to include, but not limited to the following: General, Civil, Sewer, Water, Standard Details, etc. Also, include a PDF file of all deliverables. All CD/DVDs shall be labeled with the project name, company name, contact name, phone number, and the Town of Camp Verde Project number.

9.3 BENCHMARKS

9.3.1 Benchmark Reference

All civil engineering design plans must reference on the cover sheet the published NGS monument and Town of Camp Verde benchmarks used for vertical control. Description and elevation in the required NAVD88 datum shall be shown.

9.3.2 Plan Datum Reference

All civil engineering design plans shall contain a statement certifying the horizontal and vertical datum used for all horizontal locations and elevations, which are represented in the plans. The statement shall be included on the cover sheet immediately below the benchmark.

9.3.3 Current Datum

Published Town of Camp Verde Benchmarks which reference the National Geodetic Vertical Datum of 1929 (NGVD29) are no longer acceptable for vertical control.

FIGURE 14: SURVEY DATUM REQUIREMENTS

TOWN OF CAMP VERDE SURVEY DATUM REQUIREMENTS	
COORDINATE UNITS:	International Feet
DISTANCE UNITS:	International Feet
HEIGHT UNITS:	International Feet
DATUM	
GEODETTIC (HORIZONTAL) DATUM:	North American Datum of 1983 (2011)
COORDINATE SYSTEM:	Arizona Coordinate System (State Plane)
ZONE:	Arizona Central (0202)
VERTICAL DATUM:	North American Vertical Datum of 1988 (NAVD88)
GEOID MODEL:	GEOID99 (Conus)

10 ALTERATIONS & MODIFICATIONS

10.1 INTRODUCTION

10.1.1 Purpose

- A. The purpose of this Section is to provide guidance to those seeking a formal request to allow an alteration or modification of the adopted EDSM and Town of Camp Verde supplement to MAG Specifications and Standard Details. This Section outlines submittal guidelines, processing timelines for review, fees and the decision and appeal processes for all types of alternations or modification requests. There are three types of alterations or modifications.
 - 1. Administrative Alteration or Modification
 - 2. Technical (Minor) Alteration or Modification
 - 3. Design (Major) Alteration or Modification
- B. The most appropriate time to submit a formal alteration or modification to the engineering standards request is in the preliminary stages of a project's development, in order to avoid delays to the project. Once an applicant decides to move forward with the formal request for an engineering standards alteration or modification, the applicant must follow the process outlined.
- C. Submittal of the formal request for an alteration or modification of the engineering standard does not guarantee approval. Therefore, if the applicant proceeds with design prior to Town staff providing a written determination on the deviation, is at the applicant's own risk.
- D. While a request is under review, the plan review for the project will be placed on an administrative hold until the requested alteration or modification has been completed.

10.1.2 Qualification Criteria

- A. For an engineering standards alteration or modification request to be considered, the applicant must demonstrate that there is a unique property condition causing substantial hardship. In order to qualify for an engineering standards deviation request, the following criteria must be true:
 - 1. There are unique conditions or circumstances applying to the use of the subject property in regard to:
 - a. Lot size or shape
 - b. Topography
 - c. Access control
 - d. Drainage patterns
 - e. Distance to nearest Water Service
 - f. Distance to nearest Sewer Service

- g. Unusual utility conflicts
- 2. The condition causing hardship was not created by the applicant, owner, or any previous owner of the property. The property hardship cannot be self-imposed.
- 3. Without the granting of the alteration or modification, the property cannot be reasonably used.
- 4. Certification that the alteration or modification will not be materially detrimental to persons residing or working in the vicinity, to the adjacent property owner, to the neighborhood, or public welfare in general.
- 5. State or Federal requirements or standards cannot be altered or modified by the Town
- B. It is the burden of the applicant to prove all of the above criteria true. If all of the above criteria can be proven, the Engineer of Record should contact the Public Works Division to discuss the feasibility of applying for an alteration or modification.
- C. In no case shall economic hardship qualify as criteria for evaluating an engineering deviation.

10.2 ALTERATIONS OR MODIFICATION PROCESS

10.2.1 Submittals

When deemed appropriate, the design engineer/applicant shall prepare a formal submittal to request consideration of an alteration or modification of the adopted EDSM.

10.2.2 Application & Documents

- A. All requests for alterations or modifications to EDSM shall be submitted to the Public Works Division. Applications shall be filled out completely with the printed name and signature of the current property owner and/or agent, as applicable.
- B. All requests for Alteration or Modification to EDSM shall be made with the first submittal or as soon as the issue is identified.
- C. In addition to the Engineering Standard Manual Alteration/Modification Application, the submittal shall include:
 - 1. A narrative describing the project, the proposed modification, justification for the request as defined in the EDSM, Section 10.1.2 and the section of the EDSM, which relates to the modification request.
 - 2. A site plan and an engineered plan signed by the registered professional for the proposed project.
 - 3. Specific engineering analysis needed by staff to complete the evaluation of the request to include but not be limited to, traffic studies, drainage studies, water and sewer design reports, geotechnical studies, bridge reports, structural reports, typical sections, cross sections and alternative designs.

4. Should additional information be necessary to clarify or facilitate the review of an application, the Town may request any other pertinent information deemed necessary, regardless of items already submitted for other review processes.
- D. Application for Alteration or Modification to EDSM may be submitted prior to or with the underlying development application.
- E. The Town Engineer shall determine the classification for each alteration or modification request (i.e. Administrative Modification, Technical (Minor) Modification or Design Standard (Major) Modification) based upon the complexity of the request. The Town Engineer may modify the classification after reviewing the submittal.

10.2.3 Application Completeness

- A. An application submittal shall be deemed complete only after the Town Engineer reviews the application, including all required or requested information and exhibits.
- B. The Town Engineer shall make a determination of application completeness within three (3) working days of the submittal.
 1. If an application is determined to be incomplete, the Town Engineer will contact the applicant to explain the application's deficiencies and return the incomplete submittal.
 2. No further processing of the application shall occur until the required submittal elements are provided.
 3. If the deficiencies are not corrected by the applicant within thirty (30) calendar days of notification, the application shall be considered withdrawn.

10.2.4 Justification for Request

- A. The Town Engineer shall evaluate the alteration or modification request and reach a determination based upon the information submitted and the criteria listed below:
 - Public Safety (No deviation will be granted which compromises the public's safety)
 - Function
 - Traffic Management
 - Durability/Reliability
 - Cost of Operations and Maintenance to the Town
 - Water and Sewer Service Availability
 - Environmental Quality
 - Storm Water Quality
 - Appearance
 - Planning and Zoning Ordinance Ordinance/Town Code
 - Quality of Life

- Other appropriate factors that may contribute to the council goals for the planning and development of the Town
- B. Self-imposed conditions or economic hardship shall not be used as justification to grant an alteration or modification.

10.2.5 Fees

Alteration/Modification submittals shall be accompanied by the payment of a fee. Submittals without the fee will not be accepted. Fees shall be collected in accordance with the most recent fee schedule adopted by the Town Council. If an application is denied or withdrawn, the fee payment is non-refundable.

10.3 ADMINISTRATIVE ALTERATION OF MODIFICATION

Administrative alterations or modifications are defined as a modification involving minor aspects of a particular standard that do not materially affect the functionality of the standard.

10.3.1 Review Procedure

Within five (5) working days of the determination of application completeness, the Town Engineer shall:

- A. Review and evaluate the request in light of the EDSM and other relevant information.
- B. Consult with other staff, as necessary.
- C. Render a decision to grant or deny the request.

10.3.2 Written Determination

Within five (5) working days of the determination of application completeness, the Town Engineer will provide a written decision in response to the application.

10.4 TECHNICAL (MINOR) ALTERATION OR MODIFICATION

Technical (Minor) Alteration or Modification is defined as a modification involving aspects of a particular standard that may change its general function, but not its essential function.

10.4.1 Review Procedure

Within ten (10) working days of the determination of application completeness, the Town Engineer shall:

- A. Review and evaluate the request in light of the EDSM and other relevant information.
- B. Consult with other staff, as necessary.
- C. Render a decision to grant or deny the request.

10.4.2 Written Determination

Within ten (10) working days of the determination of application completeness, the Town Engineer will provide a written decision in response to the application.

10.5 DESIGN ALTERATION OR MODIFICATION

Design (Major) Alteration or Modification is defined as a modification from the EDSM substantial enough to affect the essential function of the standard. Examples include but are not limited to:

- Sight Distances
- Vertical and Horizontal Alignments
- Street Grades
- Geometric Design (length, width, bulb radius, medians, etc.)
- Design Speed Variances
- Pavement Widths
- Crossroad Circulation
- Offset Cross Street Alignments
- Access Policy and Cross Circulation Requirements
- Surface Materials
- Asphalt or Base Course Thickness
- Rolled Curb versus Straight Curb

10.5.1 Review Procedure

Within fifteen (15) working days of the determination of application completeness, the Town Engineer shall:

- A. Review and evaluate the request in light of the EDSM and other relevant information.
- B. Consult with other staff, as necessary.
- C. The Town Engineer may obtain an independent review by a design professional with expertise in the area of the request, if deemed necessary. An additional fee from the independent reviewer may be required which will be passed on to the applicant, and a possible extension of review time may apply.
- D. Render a decision to grant or deny the request.

10.5.2 Written Determination

Within fifteen (15) working days of the determination of application completeness, the Town Engineer will provide a written decision in response to the application.

10.6 GENERAL CONDITIONS

- A. The alteration or modification of the same standard may not be submitted for evaluation more than once on the same project site.

- B. Although alteration or modification requests can be submitted until the point at which construction plans are approved, any additional plan review fees required as a direct result of submitting later in the process will be the sole responsibility of the applicant.
- C. If no appeal is filed to the Public Works Division in writing within thirty (30) calendar days, the decision shall be considered final.

11 RECORD DRAWINGS (AS-BUILTS)

11.1 INTRODUCTION

11.1.1 Purpose

The purpose of this Section is to provide record drawing submittal, review approval guidelines, and requirements for the record drawings of public infrastructure improvement projects within the Town of Camp Verde.

11.1.2 Applicability

The plan submittal, review, and approval guidelines shall apply to all public infrastructure projects which include grading, drainage, storm drain, water infrastructure, water main, reclaimed water, wastewater main, fire line, transportation or traffic infrastructure, landscaping or survey data as defined in the Town of Camp Verde's EDSM.

11.2 RECORD DRAWING PLANS

11.2.1 Record Drawing Plan Submittal

- A. At the completion of the construction of the project and after receiving "APPROVAL" of the "Public Works – Final" Inspection, the Engineer of record shall make a record drawing submittal to Town of Camp Verde Community Development Division. The Engineer of record shall submit the following items as part of the Record Drawing Submittal:
 1. One (1) mylar copy, 22" x 34", of the complete "Record Drawing" set of plans. The mylar shall have an original engineer's seal and signature and each page shall be noted as "As Built". Partial sets of plans shall not be accepted.
 2. Three (3) bond copies, 22" x 34", of the complete "Record Drawing" set of plans. Each copy shall have an original engineer's seal and signature, and each page shall be noted as "As Built". Partial sets of plans shall not be accepted.
 3. PDF file of the complete "Record Drawing" set of plans.
 4. Arizona Department of Environmental Quality (ADEQ), "Approval of Construction" (AOC), if applicable.
 5. Digital File on the Town of Camp Verde Coordinate System in conformance with EDSM, Section 9: Survey. All CAD files will be on a USB flash drive in AutoCAD (a release currently supported by Autodesk subscription plans). CAD files shall consist of base/design files on Town coordinates to include, but not limited to the following: Topography, ROW, Sewer, Water, Reclaimed Water, Drainage, Grading, Typical Sections, etc. All deliverables to include but not limited to the following: General, Civil, Sewer, Water, Reclaimed Water, Details, etc. All

CD/DVDs shall be labeled with the project name, company name, contact name, phone number, and the Town of Camp Verde project number.

6. Test data in PDF and hardcopy format (e.g. water line disinfection/chlorination, pressure and bacteria test results; low pressure air, mandrel and vacuum manhole testing results; main & lateral closed-circuit video (CCTV) inspection results). Refer to EDSM, Section 4: Water, Section 5: Reclaimed Water and Section 6: Wastewater.

Failure to submit the items listed above in conformance with the EDSM may result in the delay of the setting of a water meter on the site as well as delay of an “Approval to Operate and Release” from the Town of Camp Verde Town Engineer.

- B. The “Record Drawing” submittal shall have a company transmittal sheet listing the name and number of items submitted. The transmittal sheet shall also contain the company name, contact name and phone number.
- C. Record Drawing plans shall include the following information, if it is applicable to the project:

1. Grading & Drainage Plans

- a. Finished pad grades. An average pad grade may be used if the pad is not flat. Pad elevations shall not exceed plus 0.5 feet tolerance (plus 0.2 feet if located adjacent to an existing development). Pad elevations shall not exceed minus 0.2 feet tolerance (0.1 feet if located in a floodplain adjacent to a wash or channel)
- b. Flow line elevations of channels
- c. Hinge point elevations on all slopes and grade breaks
- d. Percentage of all slopes, flow lines and channels
- e. Catch basin grates elevation at top of grate
- f. Inverts of storm drain lines and headwalls
- g. As-built elevations shall be provided at all drainage control points (i.e. detention overflow point, tops and bottom of detention basins, drain rims, valley gutters, curbs, curb openings, flow line elevations in swales, etc.). As-built enough spot elevations to verify the design intentions are met (i.e. grade breaks, high/low points, scuppers, extreme outfall, etc.). Show the direction of drainage flow to illustrate that design intent has been met
- h. Provide calculations to verify the actual as-built volume of all detention facilities included on the as-builts, as well as a table which compares the as-built volumes with the approved, required volumes indicated on the design drawings or in the approved Drainage Report
- i. Detention calculations shall be revised to as-built condition by the engineer of record

- j. Flow line elevations and/or pipe inverts, grate elevations for catch basins, underground detention storage tanks, and all other drainage structures
 - k. Top of flood walls, retaining walls, and cutoff walls
 - l. Stations, offsets, and invert elevations for spillways and box culverts
 - m. When storm drain lines and appurtenances are included in the G&D drawings, please refer to the Storm Drain Plan record drawing requirement for additional required items
2. Water/Reclaimed Water/ Fire Line Plans
- a. Location of all valves
 - b. Main line horizontal angle points
 - c. Pipe sizes, lengths and materials
 - d. Blow-offs and meter boxes
 - e. Horizontal and vertical separation from existing and new utilities and drainage culverts/storm drain
 - f. Street centerline station and offset dimension to:
 - i. All fire hydrants and fittings (e.g. valves)
 - ii. Main at all changes in alignment
 - iii. All horizontal control points (e.g. centerline intersects, PC, PT)
 - g. Centerline station and offset to each service tap, size of tap and meter
 - h. Note centerline station, offset and elevations to all changes in vertical alignment (e.g. dips, bends, etc. required to avoid conflicts with other utilities)
 - i. The drawings must clearly indicate the specific points of reference. No dimensioning from points of curvature or tangency is acceptable for record drawing purposes. In all cases where the pipeline is constructed within, or parallel in close proximity with the ROW, all stationing and dimensioning must be from the nearest appropriate monument line and monument line intersection
 - j. When water services are not installed perpendicular to the water main, both the location of the tap at the main and the distance of the meter set from the nearest side property line of the lot must be shown
 - k. On phased projects, the phase lines must be clearly shown on the key map and on the plan and profile sheets, and their locations clearly identifiable. Actual pipe end locations relative to phasing lines must be shown by dimensioning of stationing
 - l. A complete list of all materials installed and abandoned must be shown. The specific size and material type of each pipeline installed must be shown at every construction reference to that pipe. Any changes to the record drawing must be reflected on the materials list
3. Sewer Plans
- a. Manhole rim and inverts

- b. Pipe sizes and lengths
- c. Recalculated pipe slopes
- d. Location of cleanouts and individual service wyes
- e. Separation from existing/newly installed water mains, reclaimed water mains and culverts
- f. Street centerline station and offset dimension from street centerline to main at manholes
- g. Sewer line stationing at centerline of each service tap at 90 degrees to main; if not installed at 90 degrees to main, station and offset to end of each service tap
- h. New manholes built on existing lines require showing its station from the nearest existing downstream manhole and its distance to the nearest existing upstream manhole
- i. Where construction begins with removal of an existing pipe plug of cleanout, 0+00 stationing always begins at the nearest existing downstream manhole. Pipe length measurement and stationing is always from the centerline of the downstream manhole to the centerline of the upstream manhole or cleanout. Do not give partial pipe lengths in plan of profile at match lines. Always indicate the distance between manholes or to cleanouts or pipe ends
- j. All as-built adjustment to manhole, cleanout and pipe information shall be shown on both plan and profile, and repeated on every sheet that refers to the same information
- k. A complete list of all materials installed and abandoned must be shown. The specific size and material type of each pipeline installed must be shown at every construction reference to that pipe. Any changes to the record drawing must be reflected on the materials list
- l. On phased projects, the phase lines must be clearly shown on the key map and on the plan and profile sheets, and their locations clearly identifiable. Actual pipe end locations relative to phasing lines must be shown by dimensioning of stationing

4. Paving Plans

- a. Top of curb, flow line, and pavement centerline elevations at all grade breaks, Points of Tangency (PT), Points of Curvature (PC), Beginning Curb Return (BCR), and Ending Curb Return (ECR), valley gutters, spandrels at intersections, plus any other location necessary to adequately show drainage
- b. Percentage of slope
- c. Valley gutter inverts
- d. Edge of pavement on rural road sections
- e. Locations of traffic signage, signals, poles and cabinets

- f. Station for all grade breaks
 - g. Back of curb offset dimension at all changes in alignment
 - h. Survey monuments: as-built installation and provide Northing/Easting to the hundredth of one foot. For street monuments, provide top of monument as-built elevation in addition
 - i. Distance from monument line to back/face of curb, edge of pavement, and sidewalk; show on plan view or typical detail for street section
 - j. Beginning and ending stations and elevations for all traffic calming devices
 - k. Stations, offsets, and invert elevations for spillways and box culverts
 - l. Flow line elevations and/or pipe inverts, grate elevations for catch basins, underground detention storage tanks, and all other drainage structures
- 5. Traffic Signal Plans
 - a. Street centerline station and offset dimension to all fixture poles, cabinets, boxes, or other signal related furniture
 - b. Horizontal location of conduit along with elevations to top of conduit
- 6. Signing & Striping Plans
 - a. Street centerline station and offset dimension to all signage, painted arrows, wording and symbols
 - b. Face of curb dimensions to all striping
- 7. Storm Drain Plans
 - a. Street centerline station and offset dimension to the main at all changes in alignment and/or changes in grade
 - b. Street centerline station and offset dimension to all structures and changes in alignment
 - c. Top and invert elevations for all structures
 - d. Drainage pipe inverts
 - e. Finish elevation for catch basins
 - f. Invert elevations of box culverts
 - g. Length of headwall wing
 - h. Length of catch basin wings
- 8. Landscaping
- 9. Street Light Plans
 - a. Record drawings for streetlights are required to have an Arizona Public Service (APS) ID number on each streetlight noted on the plan
 - b. Street centerline stationing and offsets for streetlights

- D. Benchmark locations and elevations must be shown on the coversheet.
- E. The following items must be shown on the cover sheet by the Town of Camp Verde department responsible for construction inspection:
 - a. Name of Contactor
 - b. Permit number {identify phase if applicable}
 - c. Date of approval by Town of Camp Verde Inspector
 - d. Name of Town of Camp Verde Inspector who witnessed the construction
- F. Record Drawing plans shall be reviewed and approved by the Fire Department, Utilities Division, and Town Engineer.
- G. Record Drawings shall be signed and sealed by a Registered Professional Engineer with the “AS-BUILT CERTIFICATION” approval block as shown below on Town of Camp Verde As-Built Certification 125CV

12 APPENDIX

TOWN OF CAMP VERDE STANDARD DETAILS